# California Department of Transportation Division of Maintenance

### Structure Maintenance and Investigations

 $B_{\text{RIDGE}}$ 

INSPECTION

Records

I NFORMATION

System

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Records for "Confidential" bridges may only be released outside the Department of Transportation upon execution of a confidentiality agreement.

#### INDEX TO PLANS

#### PILE DATA - CIDH CONCRETE PILES

SHEET NO.	TITLE
1 2 3 4 5 6 7 8 9 10 11 11 11 11 11 11 11 11 11 11 11 11	GENERAL PLAN INDEX TO PLANS DECK CONTOURS FOUNDATION PLAN NO. I FOUNDATION PLAN NO. 2 ABUTMENT LAYOUT ABUTMENT DETAILS RETAINING WALL TYPE I(MOD) DETAILS 96.0 CIDH CONCRETE PILE BENT DETAILS NO. I BENT DETAILS NO. 2 BENT DETAILS NO. 3 TYPICAL SECTION GIRDER LAYOUT NO. 1 GIRDER LAYOUT NO. 2 GIRDER LAYOUT NO. 3 GIRDER LAYOUT NO. 3
18 19 20 21 22 23 24 25 26 27 28 29	GIRDER REINFORCEMENT NO.1 GIRDER REINFORCEMENT NO.2 4*0 COMMUNICATION CONDUIT PTFE BEARING PAD ABUTMENT JOINT SEAL DETAILS STRUCTURE APPROACH TYPE (N3OS) STRUCTURE APPROACH DRAINAGE DETAILS PC P/S CONCRETE DECK PANELS FOR P/S BOX GIRDERS LOG OF TEST BORINGS 1 OF 4 LOG OF TEST BORINGS 3 OF 4 LOG OF TEST BORINGS 4 OF 4

	Local	lon	Diometer	Design Loading (Service Load)	Nominal Resistance Compression	Cut-Off Elevation	Specified Tip Elevation
	Abut I	Abutment	16°	70 Tons	280 Kips		245.0
LEFT		Ref.Wall H-IO'	16'	70 Tons	280 Kips		250.0
BRIDGE	1	Ref.Wall H-6'	16*	70 Tons	280 Kips	_	254,0
		Rel.Wall H-4"	<i>16</i> °	70 Tons	280 Klps		258.D
		Bent 2	96'	1680 Tons	6720 Klps	267.5	204.0
		Bent 3	96°	1893 Tons	7572 Klps	267.D	183.0
		Bent 4	96'	2058 Tons	8232 Klps	257.0/2	<i>172.</i> 0
		Bent 5	96'	1970 Tons	7880 Kips	258.5₫	<i>184.</i> 0 <i>∧</i>
		Bent 6	96"	1805 Tons	7220 Klps	256.0	184,0
	Abut 7	Abulment	16'	70 Tons	280 Klps	_	251.0
	1	Ref.Wall H-IO	16°	70 Tons	280 Klps		256.0
		Ref.Wall H-6'	16'	70 Tons	280 Kips		260.0
		Ret.Woll H-4"	<i>16</i> '	70 Tons	280 Klps		262.0
	Abul /	Abutment	16*	70 Tons	280 Klps	<del></del>	245.0
RIGHT		Ref.Woll H-IO	16'	70 Tons	280 Kips		250.0
BRIDGE		Ret.Wall H-6'	16"	70 Tons	280 Klps		254.0
		Bent 2	96*	1710 Tons	6840 Klps	264.5	194.0
		Bent 3	96'	1895 Tons	7580 Klps	265.0	75.0
		Bent 4	96	2085 Tons	8340 Klps	256.0	720
		Bent 5	96'	2048 Tons	8192 Klps	263.0/2	187.0 A
		Bent 6	96"	2010 Tons	8040 Klps	248.5	177.0
	Abul 7	Abutment	16*	70 Tons	280 Kips		251.0
		Ref.Wall H-IO	<i>16</i> '	70 Tons	280 Kips		256.0
		Ref.Wall H-6'	<i>16</i> °	70 Tons	280 Klps		260.0
		Ret.Wall H-4"	<i>16</i> °	70 Tons	280 Klps	_	2640

#### STANDARD PLANS DATED JULY 1992

A62C	LIMITS OF PAYMENT FOR EXCAVATION AND BACKFILL-BRIDGE
BO-5	BRIDGE DETAILS
BO-13	BRIDGE DETAILS
82-3	IG CAST-IN-DRILLED-HOLE CONCRETE PILE
B3-i	RETAINING WALL TYPE I
83-8	RETAINING WALL DETAILS NO. I
B7-I	BOX GIRDER DETAILS
B7-6	DECK DRAINS TYPES D-1 & D-2
B7-10	UTILITY OPENING BOX GIRDER
88-5	CAST-IN-PLACE PRESTRESS GIRDER DETAILS
BII-53	CONCRETE BARRIER TYPE 25

## AS BUILT PLANS

Contract No. 06-342604 Contractor Bence Resident Engineer L. HICKINDOTUM Date of Completion 11(9)

1	308,17	13	302.51	25	307.46	37	310.06	49	303.28
2	308,12	14	301.94	76	306.73	38	309.94	50	303.26
3	308.11	15	301.30	27	306.07	39	309,80	51	303.02
4	307,94	16	301.29	28	305,19	40	309.59	52	310.08
5	307.75	17	301,07	29	304.32	41	309.36	53	310,91
_	2-0 17	1.00	200 00	2.	2022	4.0	700 01	-,	

\* COPPER NAIL FINAL ELEVATIONS

3	308.11	15	301,30	27	306.07	39	309,80	51	303.02		
4	307,94	16	301.29	28	305.19	40	309.59	52	310.98		
5	307.75	17	301,07	29	304.32	41	309.36	93	310,91	61	307.43
6	307.47	18	300.99	30	303.30	42	308.97	54	3(0.8)	6Z	306.08
7	307.16	19	308.93	3(	302.69	43	308.20	95	310,70	63	304.91
8	306.61	70	308,92	32	302.06	44	307.41	56	310.62	64	303.99
9	305.08	24	308.78	33	302.05	45	306.53	57	310,42	65	303.23
lo	305,25	22_	308.56	34	301.80	46	305,5%	58	310.18	66	302.55
11	304,39	23	308.27	35	310.07	47	304.CA	Sq	309.72	67	302,54
12	303.51	24	307.96	36	310.07	40	303.99	60	308.70	68	302.19

DIS T. COUNTY 06 Fre 41, 99 REGISTERED ENGINEER - CIVIL 1-22-96 PLANS APPROVAL DATE

The State of Colifornia or its officers or agents shall not be responsible for the accuracy or completeness of electronic copies of this plan sheet.

#### GENERAL NOTES LOAD FACTOR DESIGN

BRIDGE DESIGN SPECIFICATIONS DESIGN:

(1983 AASHTO with Interims and Revisions by CALTRANS)

includes 35 psf for future wearing surface.

LIVE LOADING:

SEISMIC LOADING: HS20-44 and alternative and permit design load.

Peak Rock Acceleration = 0.ig Depth of Alluvium > 150 ft.

REINFORCED

CONCRETE: fy = 60,000 pst

f'<sub>c</sub> = 3,250 ps!

Transverse Deck Slabs (Working Stress Design)

 $f_5 = 20,000 \text{ psi}$ 

f<sub>c</sub> = 1,200 psl

n = 10

PRESTRESSED

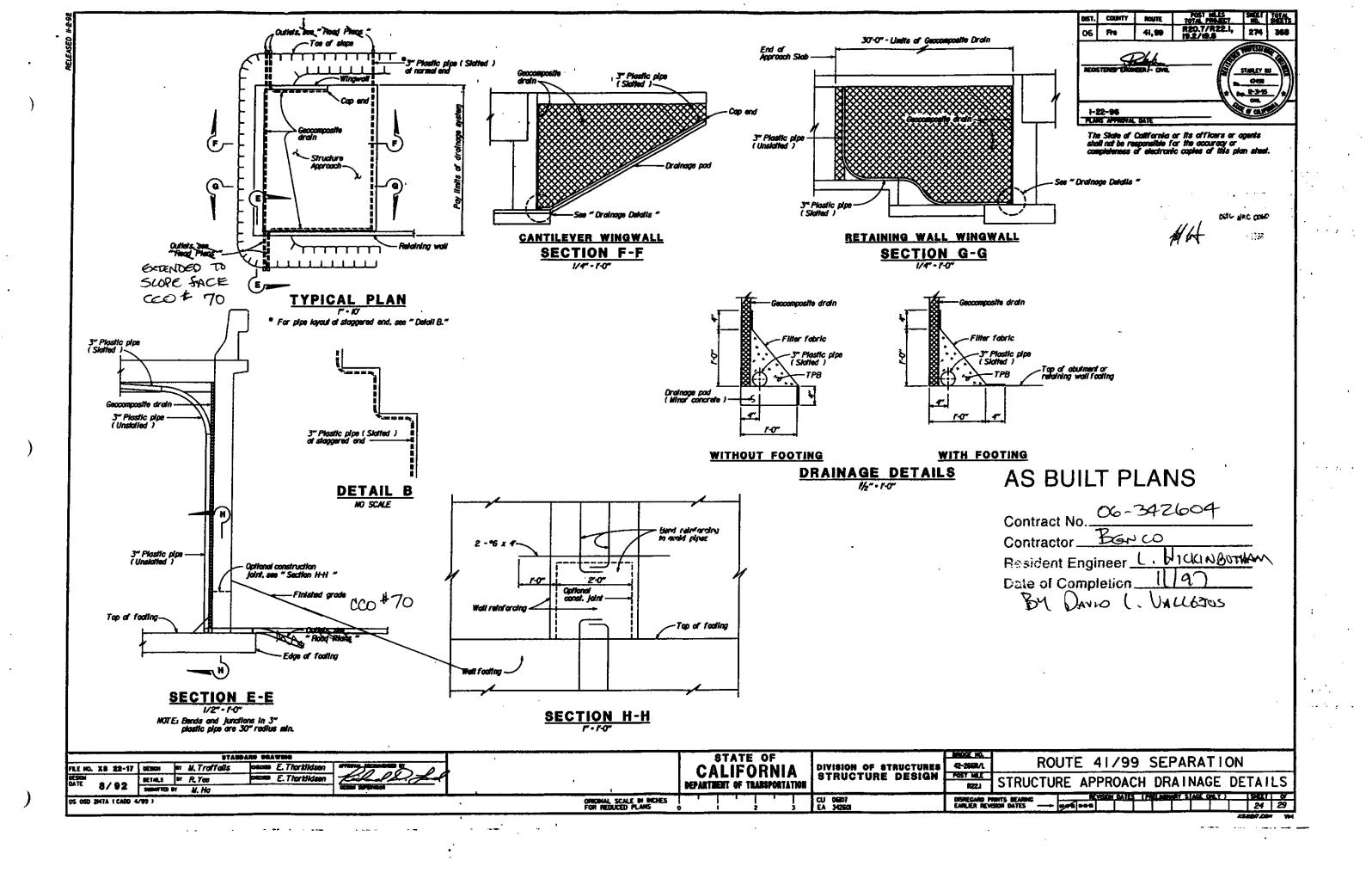
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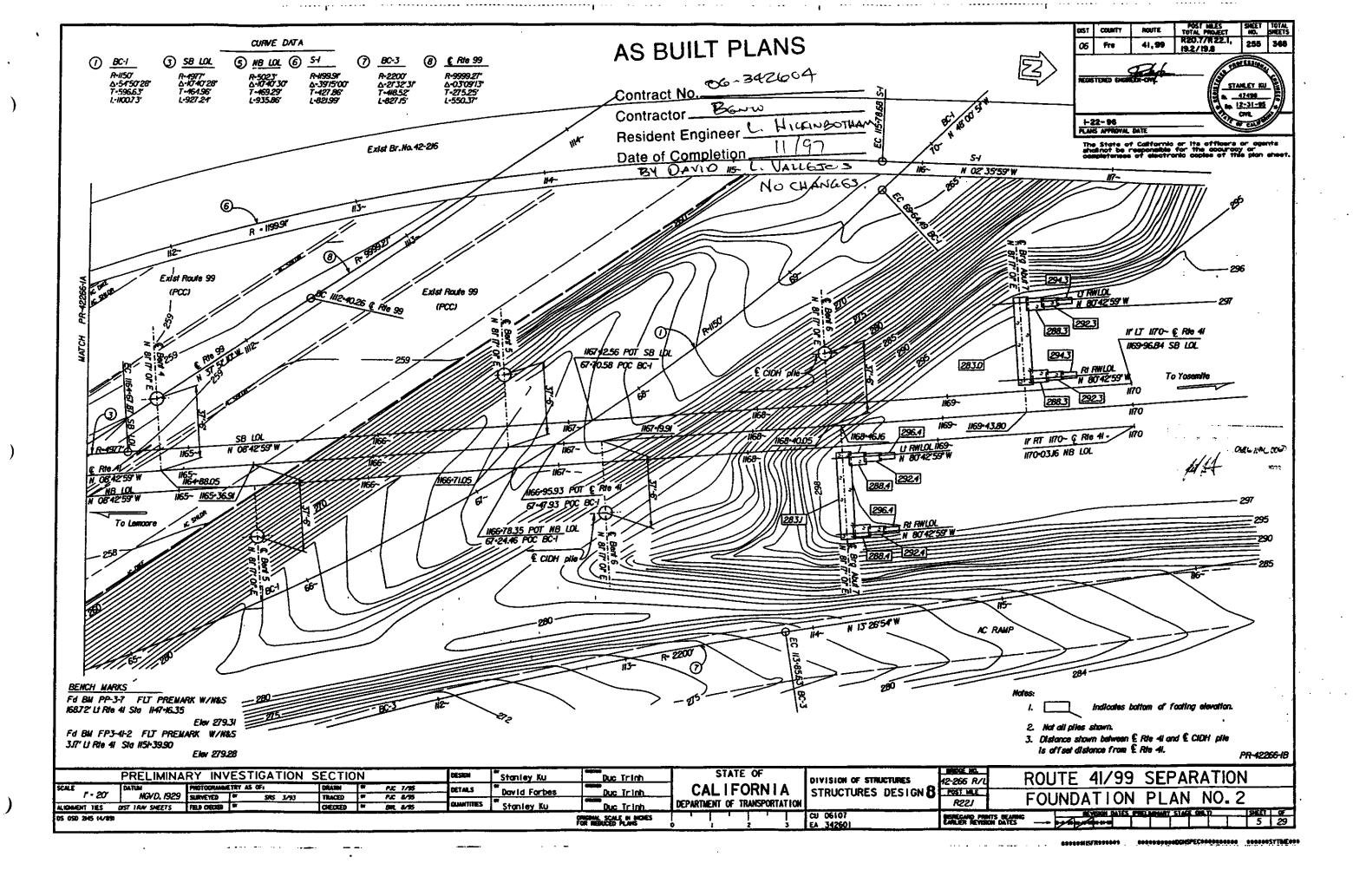
				T
$\triangle$				
<b>/2</b> \	8-8-96	Changed Elevation	DL	ОН
$\triangle$	6-19-96	Changed Elevation	SK	ÐL
MARK	DATE	DESCRIPTIONS	BŸ	ČH'D
		REVISIONS		

CONTRACT CHANGE ORDER NO. 14
SHEET 2 OF 2

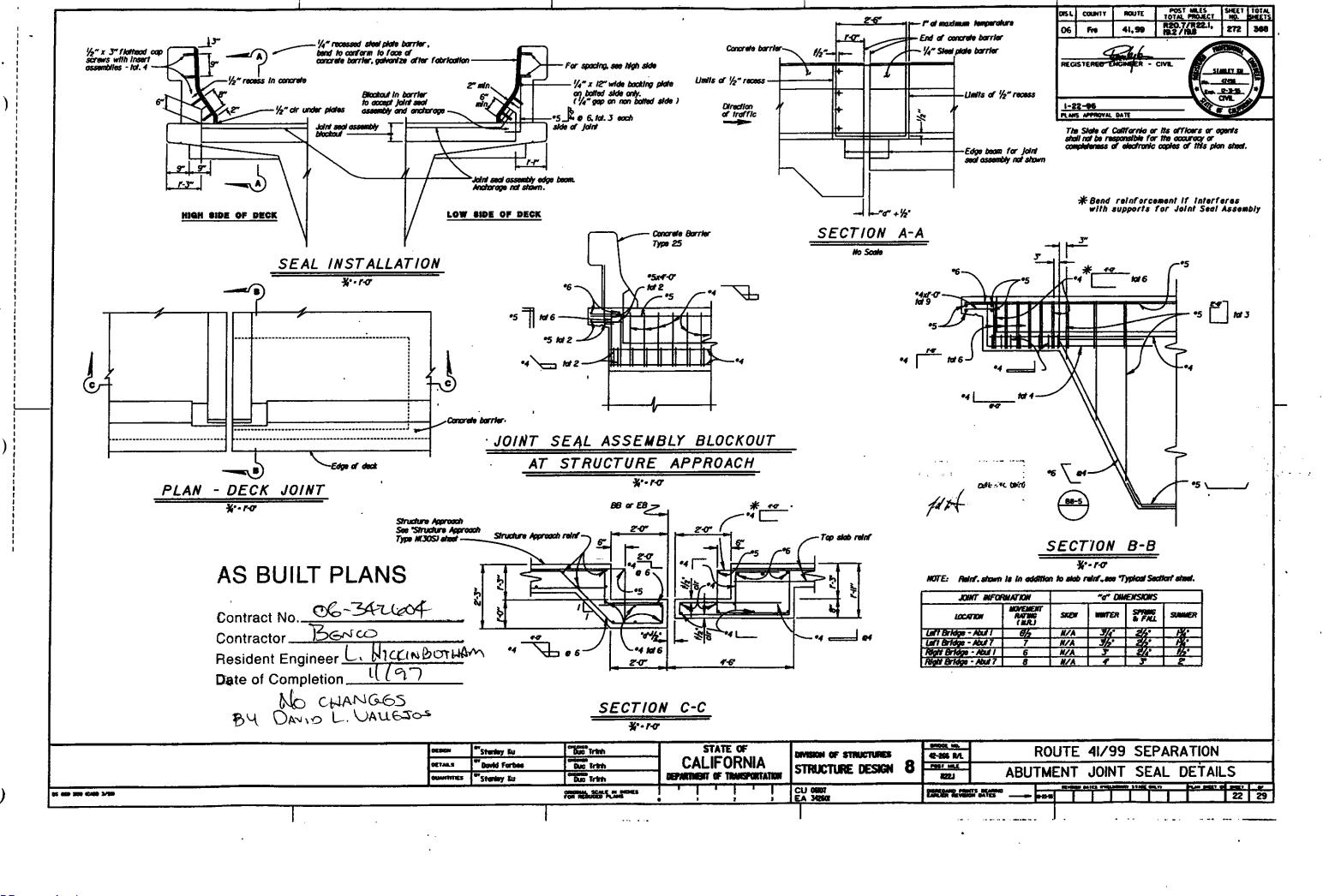
	DESIGN	lav -	Duc Trinh	STATE OF DIVISION OF STRUCTURES		42-266 R/L	ROUTE 41/99 SEPARATION				
ŀ	QUANTITIES	Roberto Elm <sup>av</sup> Stanley Ku	Duc Trinh  CHECKED  Duc Trinh	DEPARTMENT OF TRANSPORTATION	STRUCTURE DESIGN 8	POST MILE R22J	INDEX TO PLANS				
*			OFIGINAL SCALE IN INCHES	, , , , , , , , , , , , , , , , , , , ,	CU 0607	DISREGARD PRIN	ITS BEARING NEWSYCH DATES IPPELANMART STADE O'R TI IT AM SHPET ID SHEET OF				

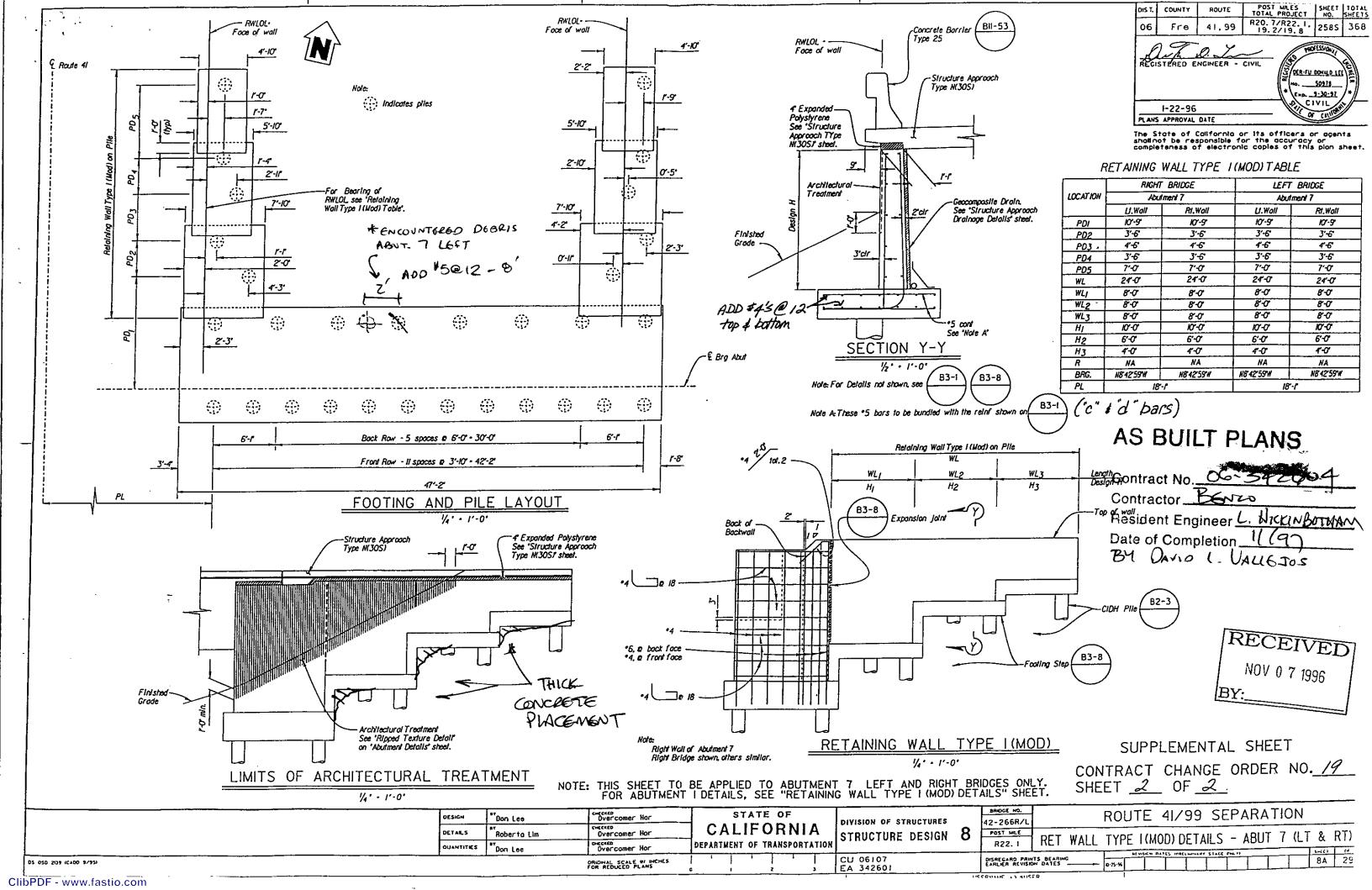
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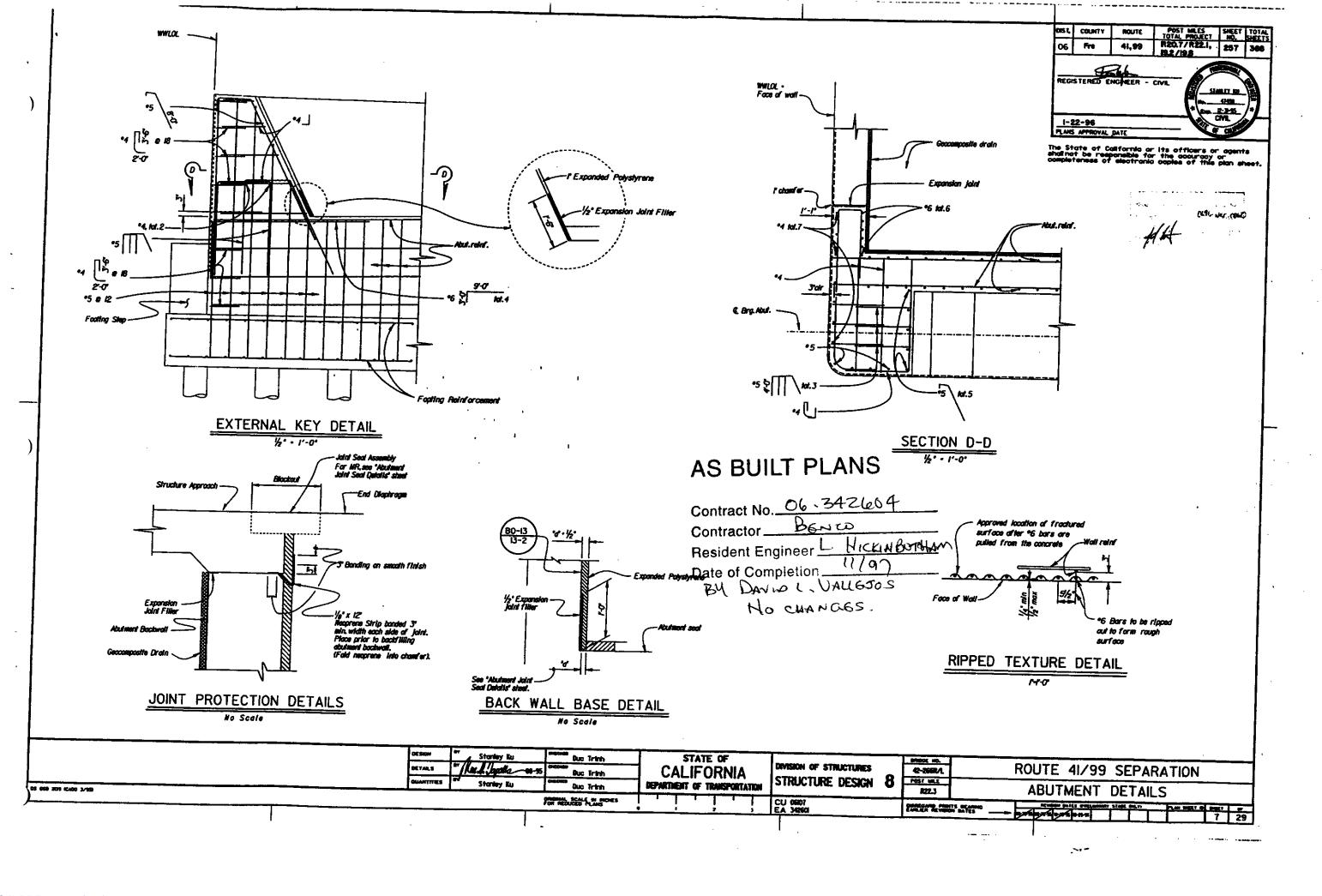


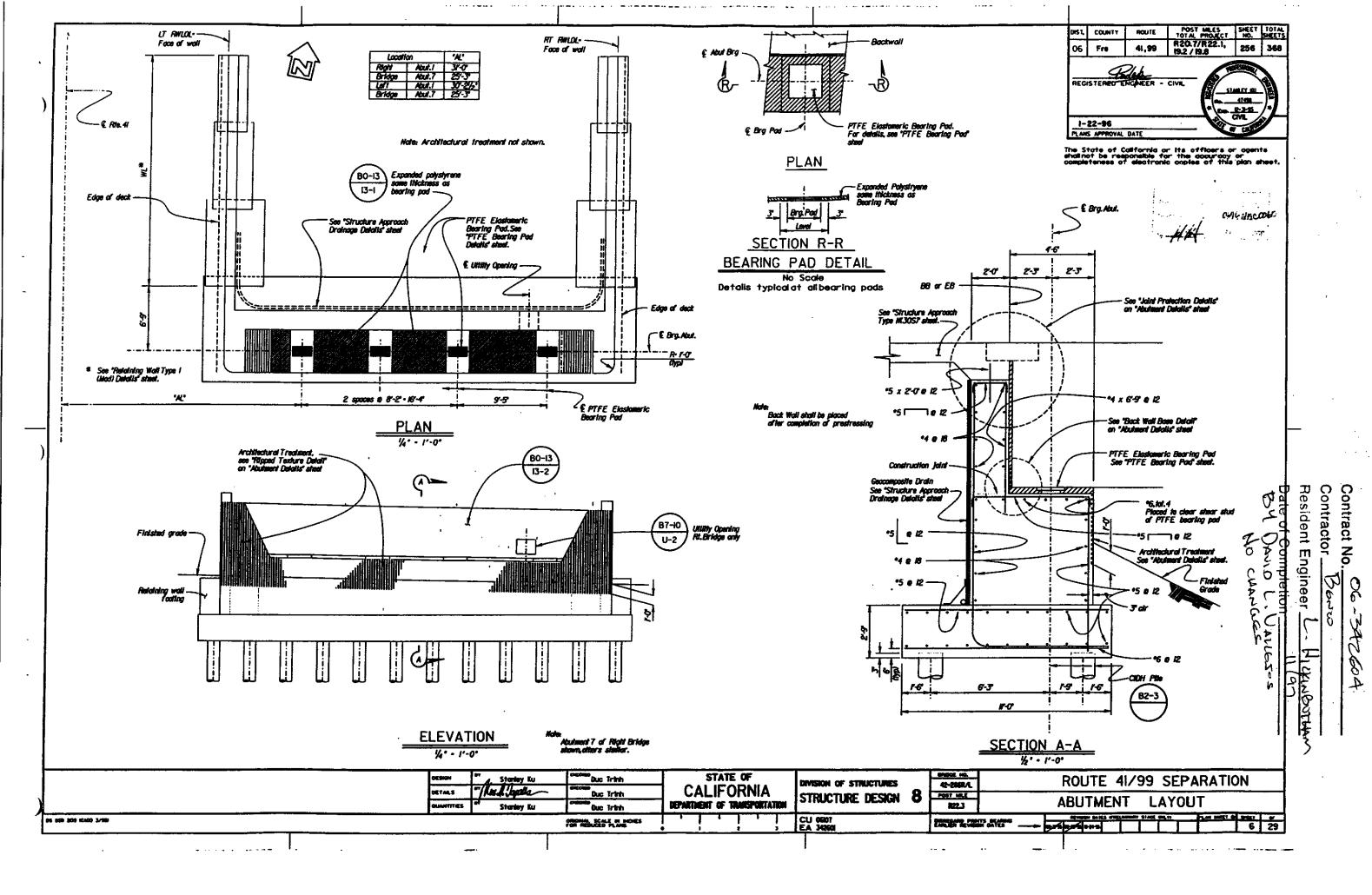


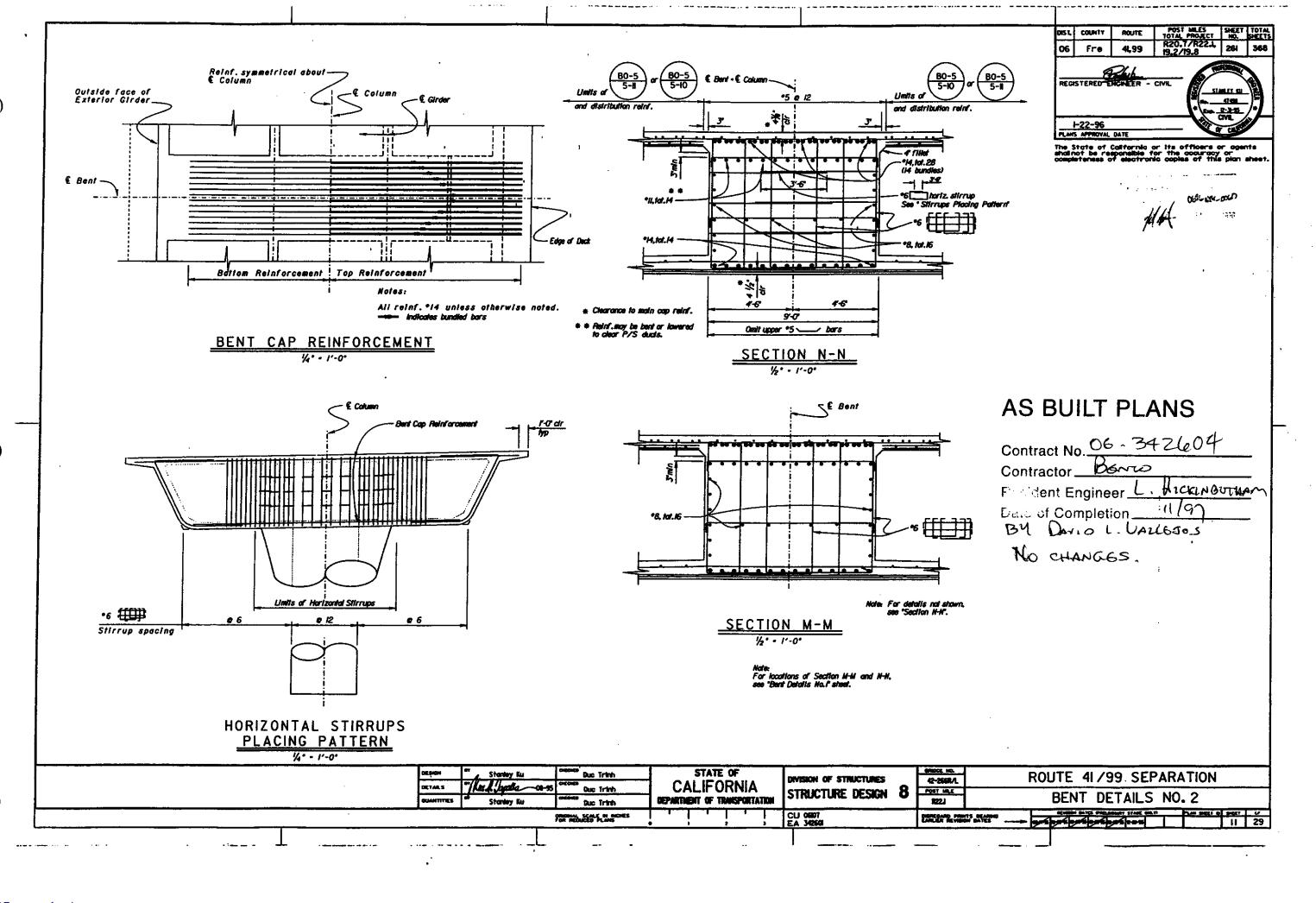
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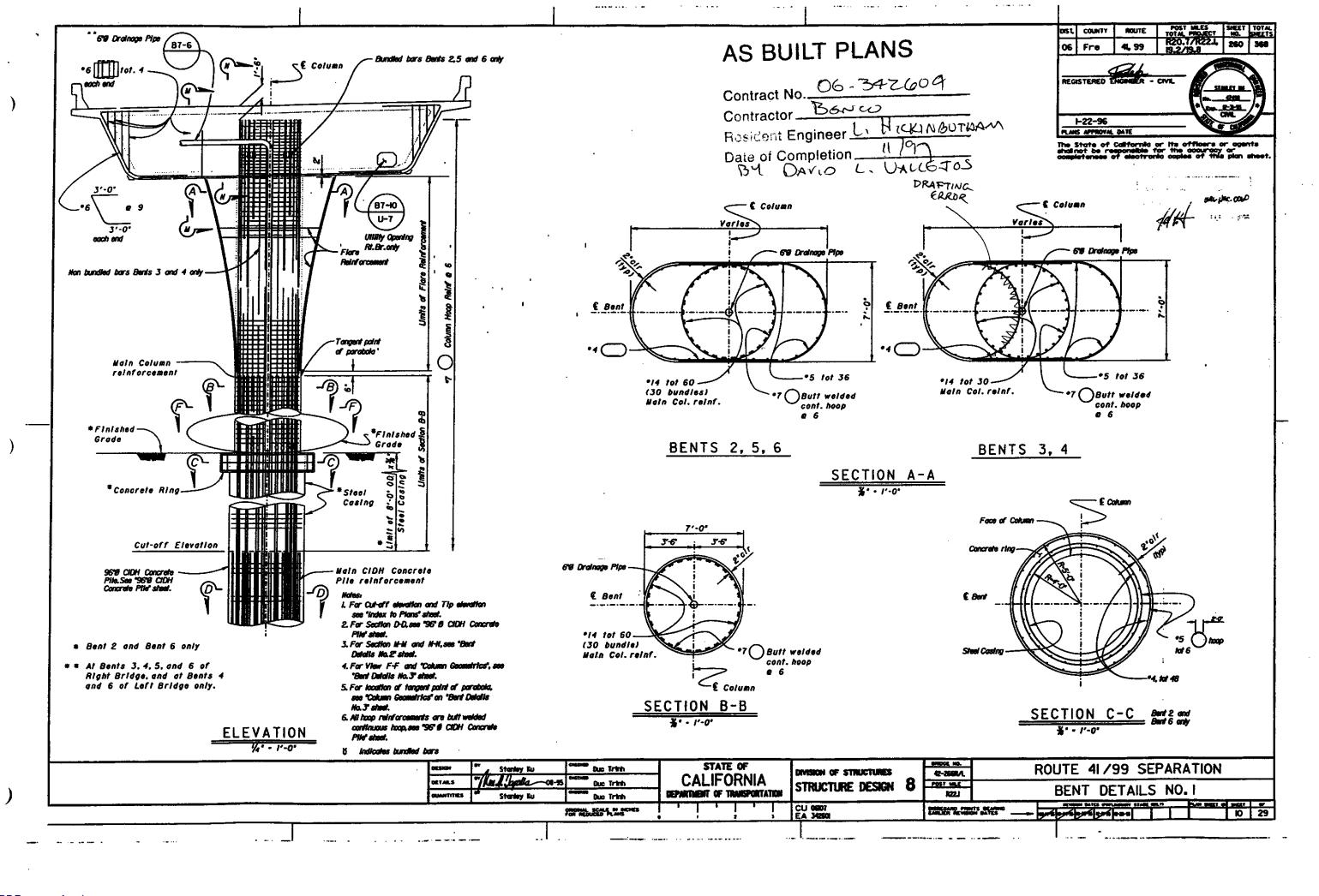


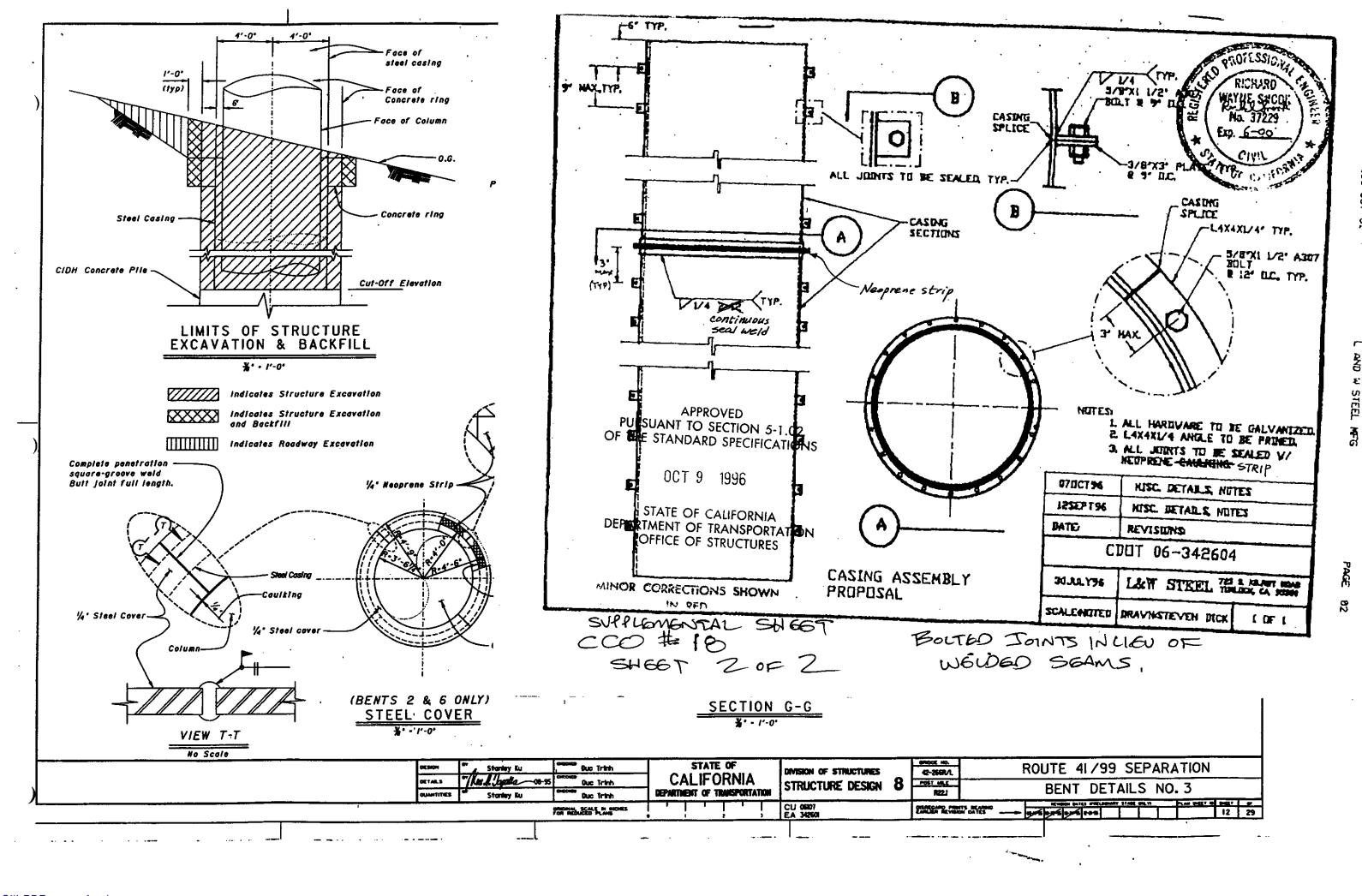




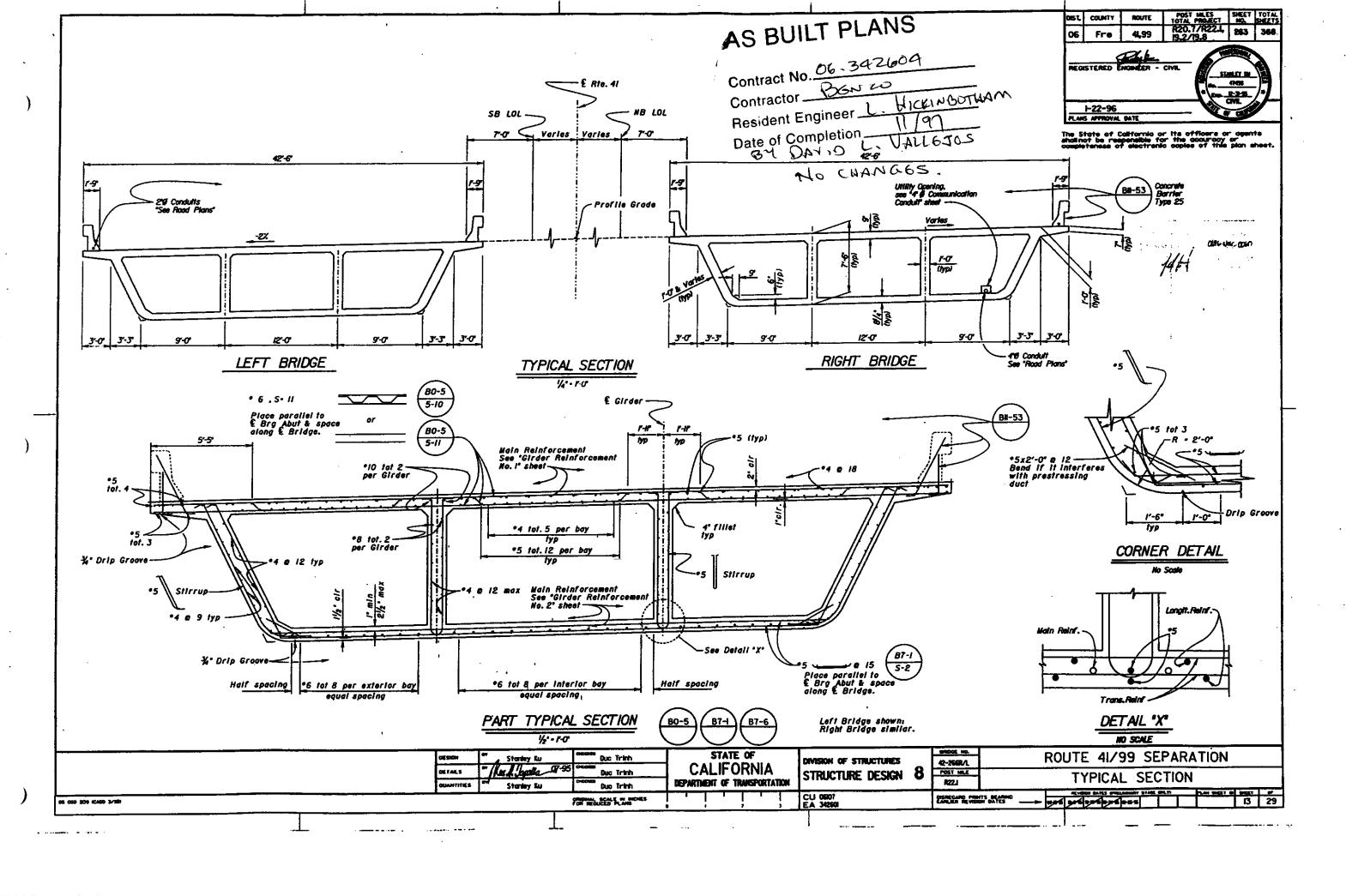


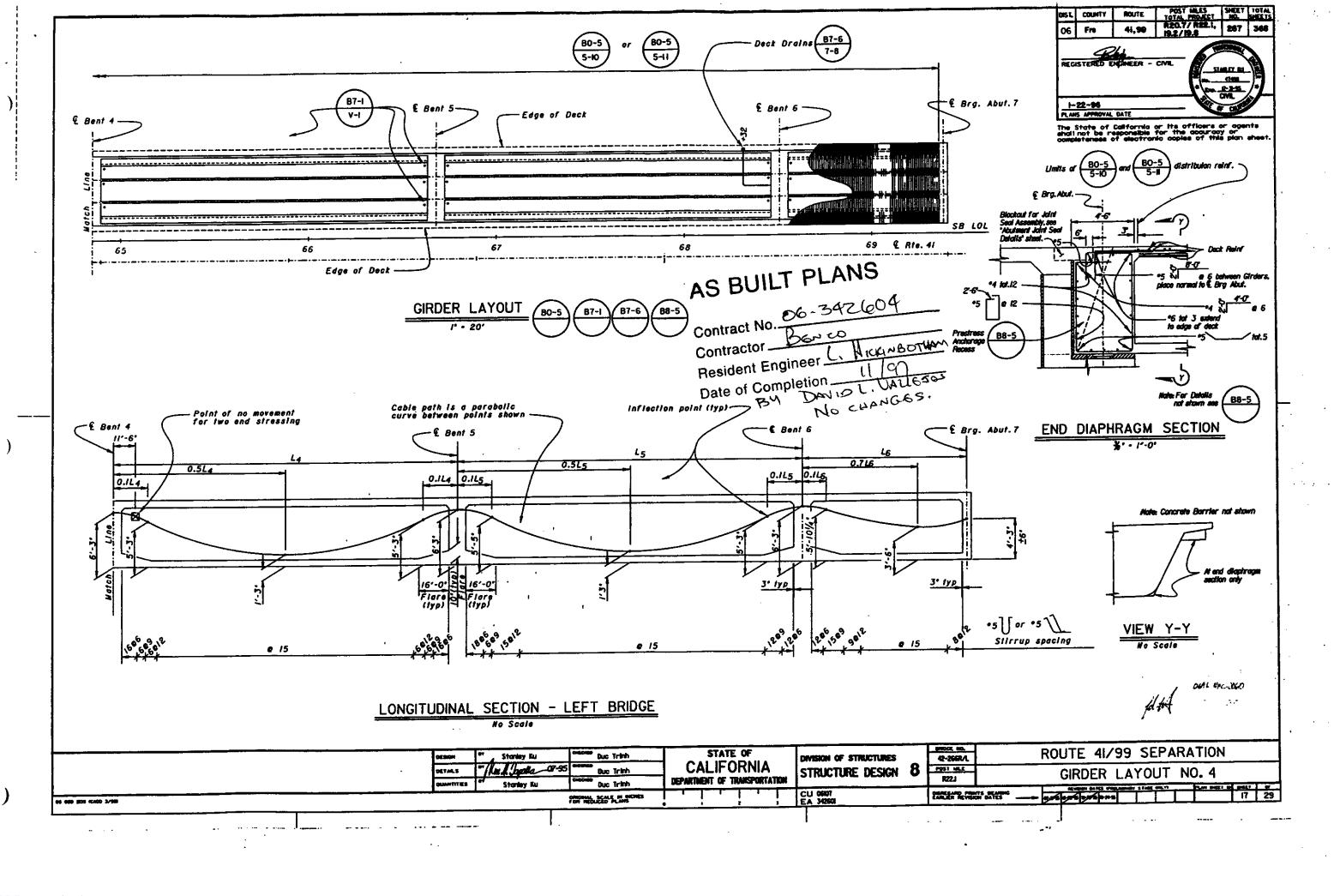


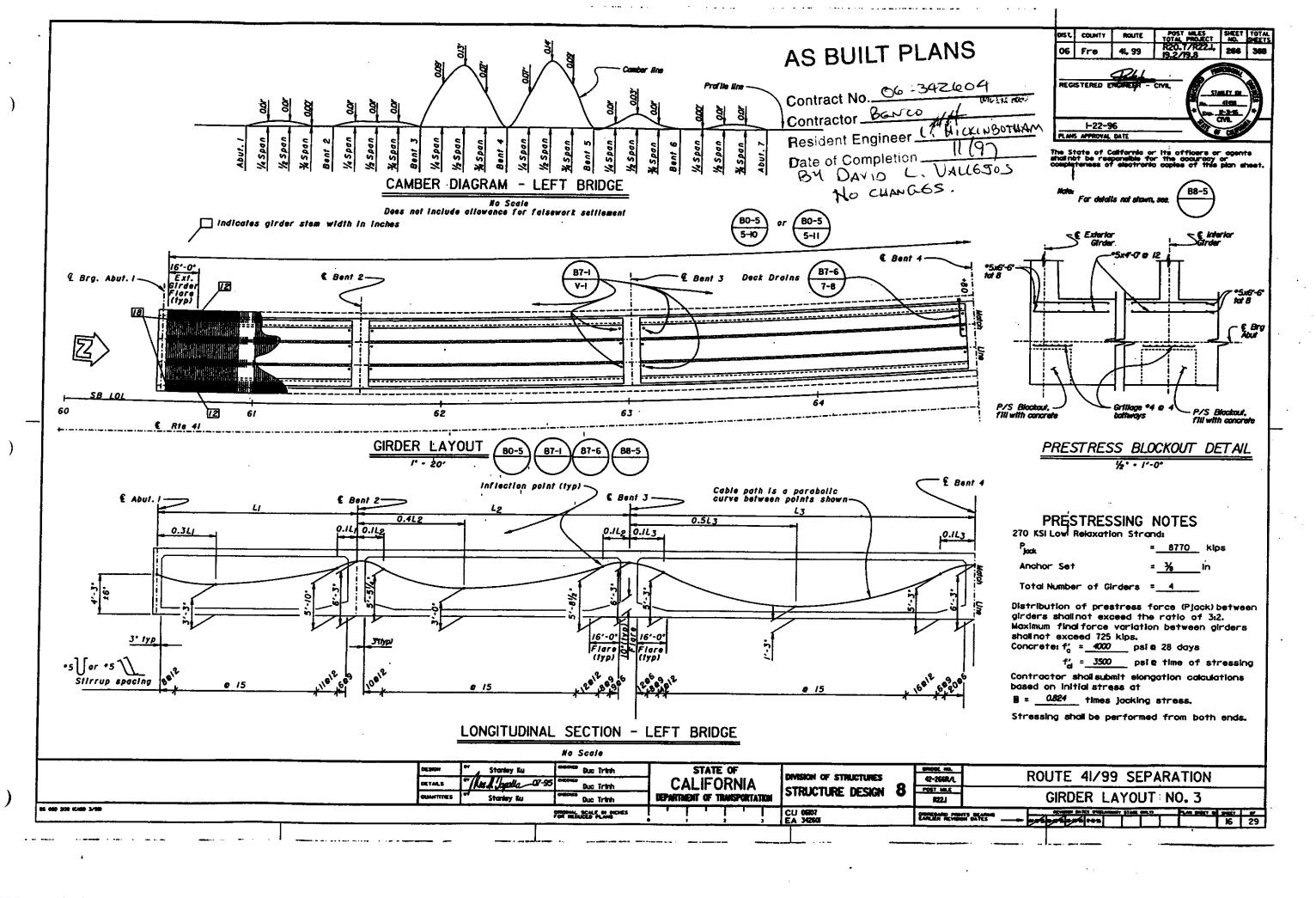


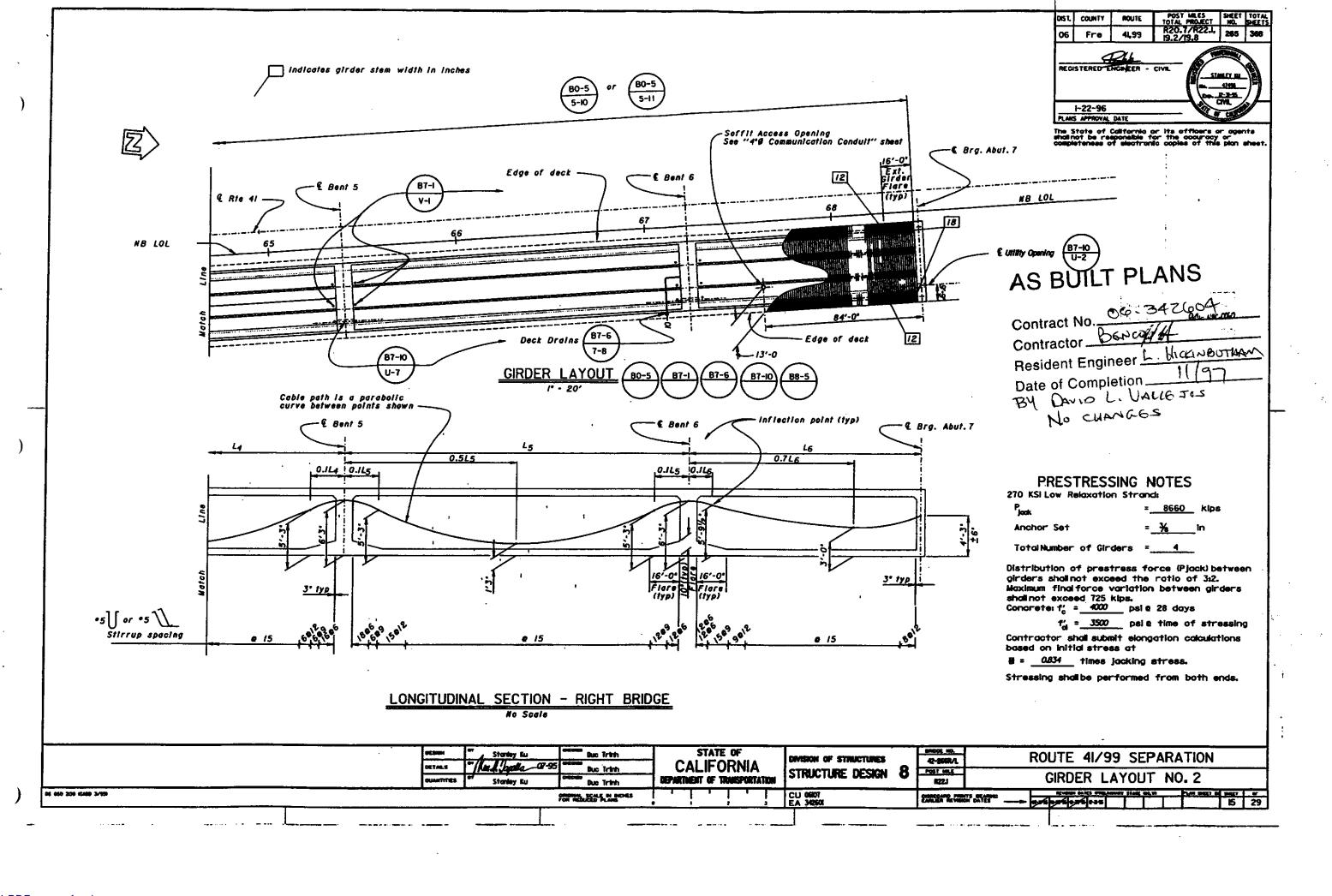


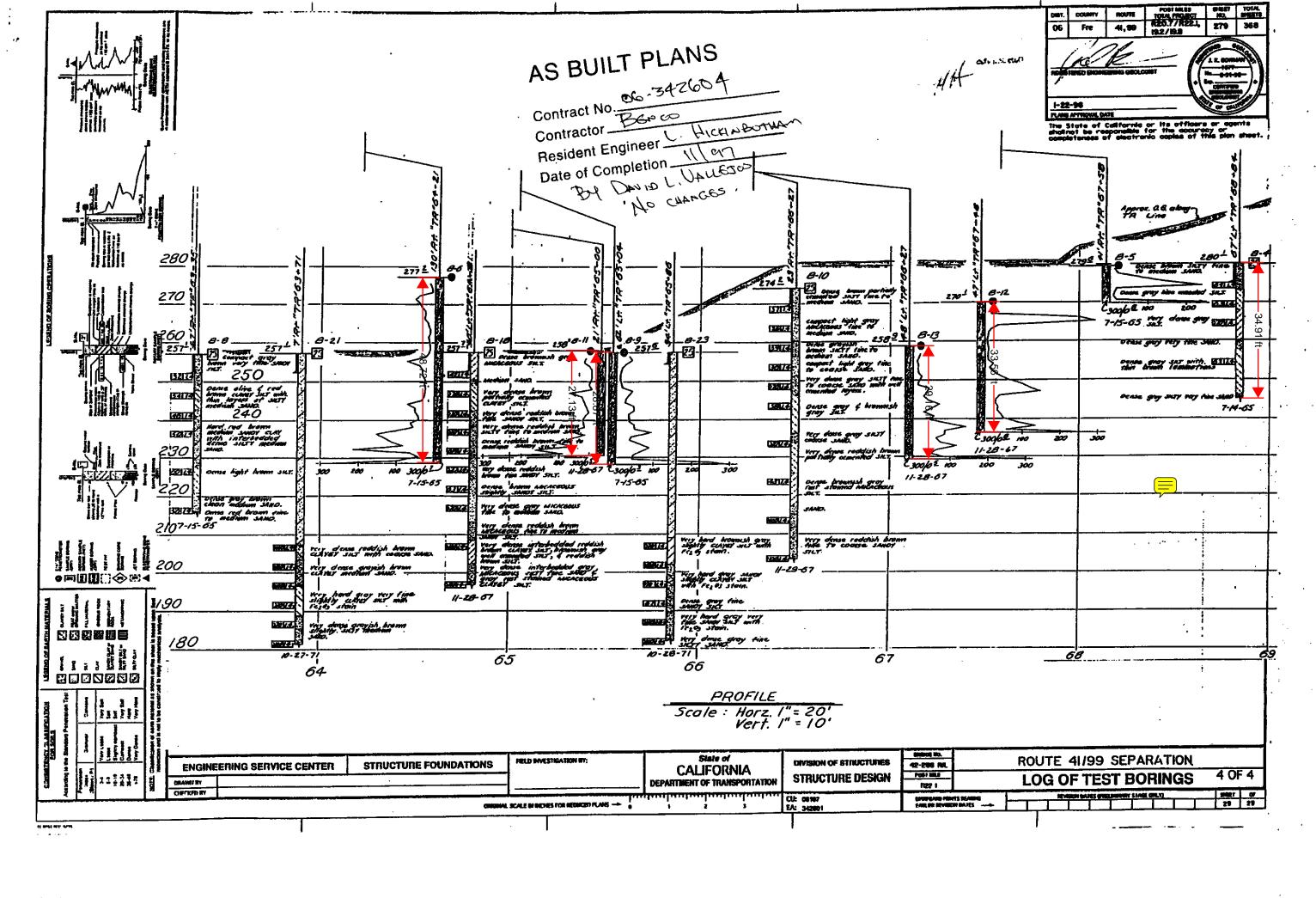
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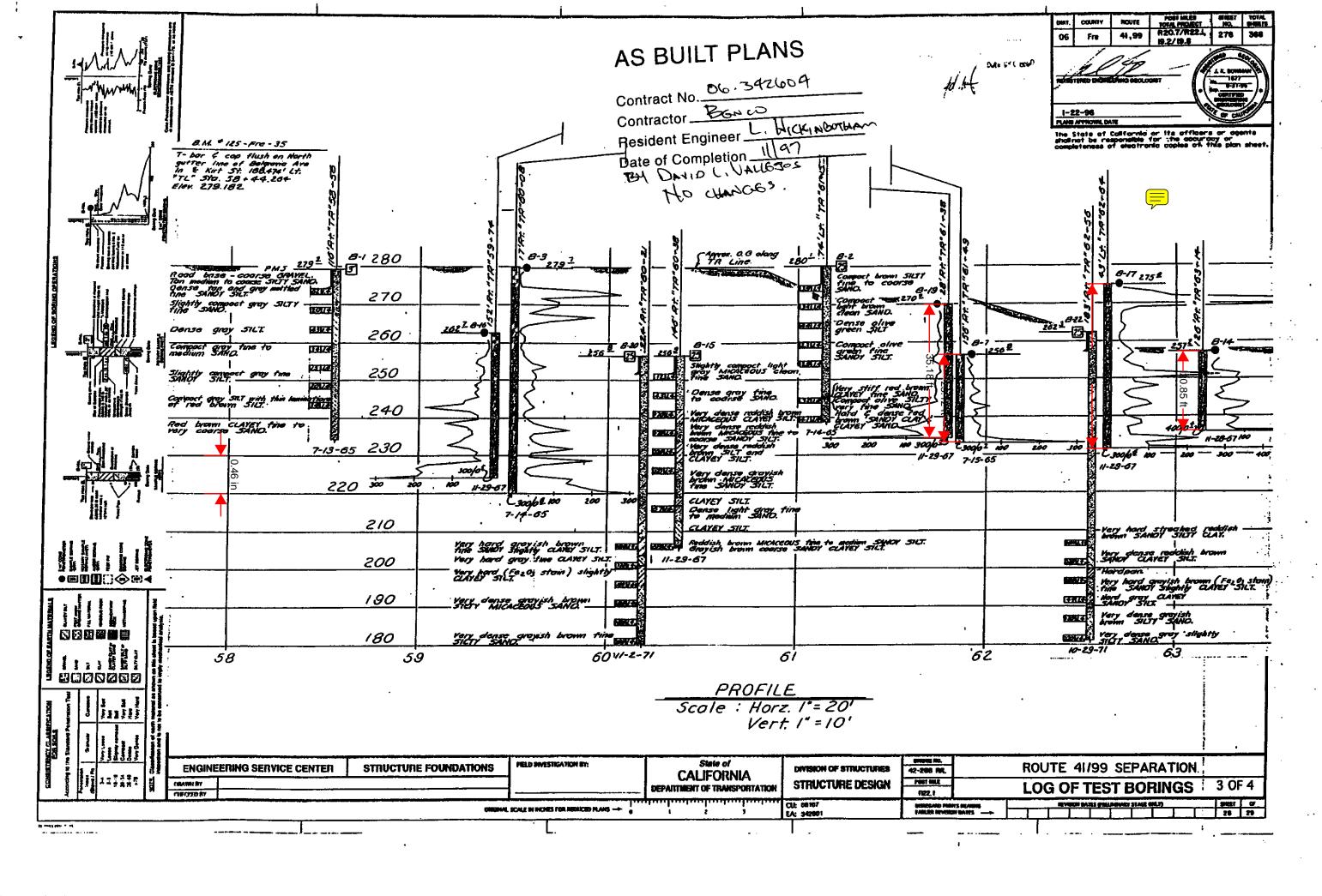


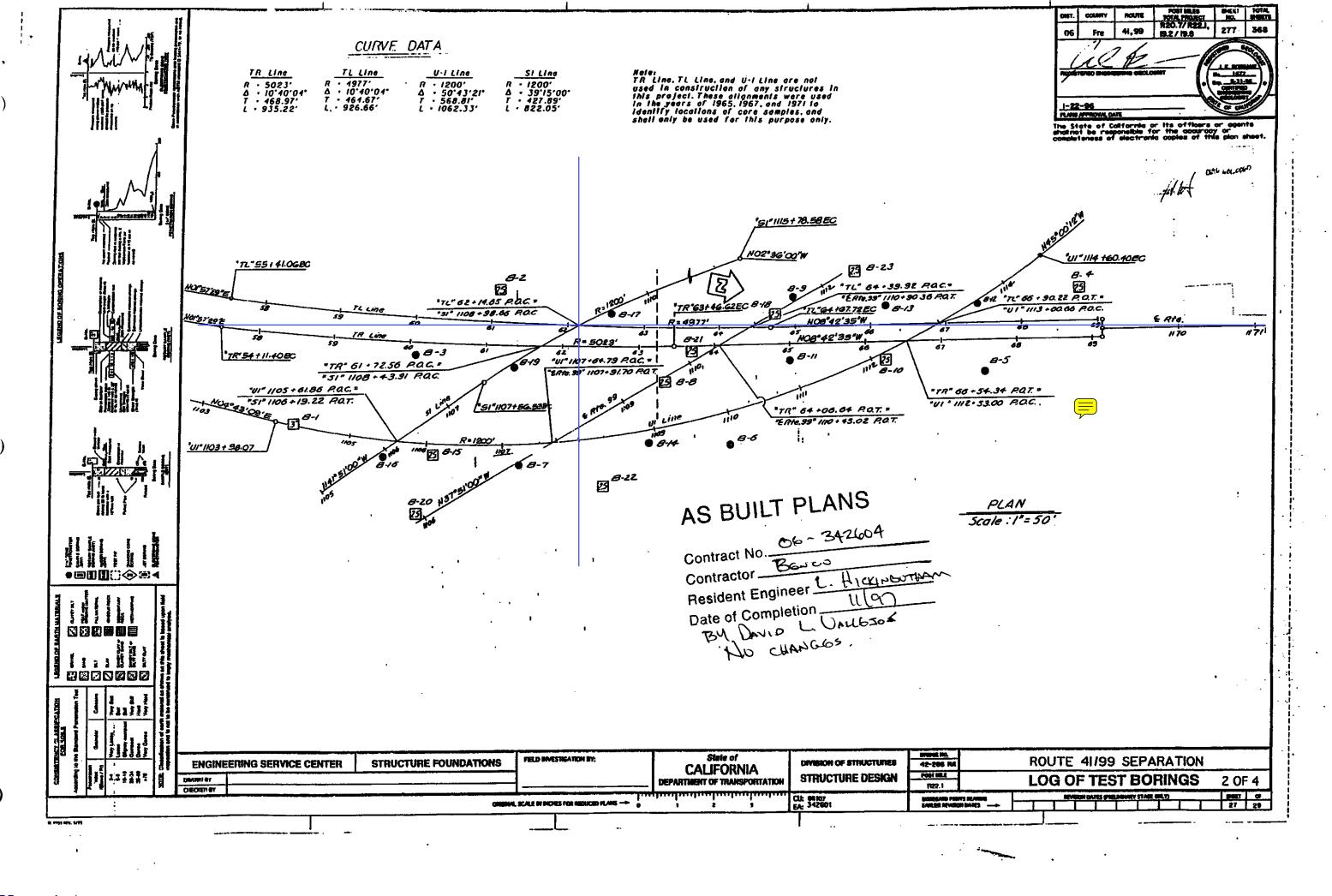


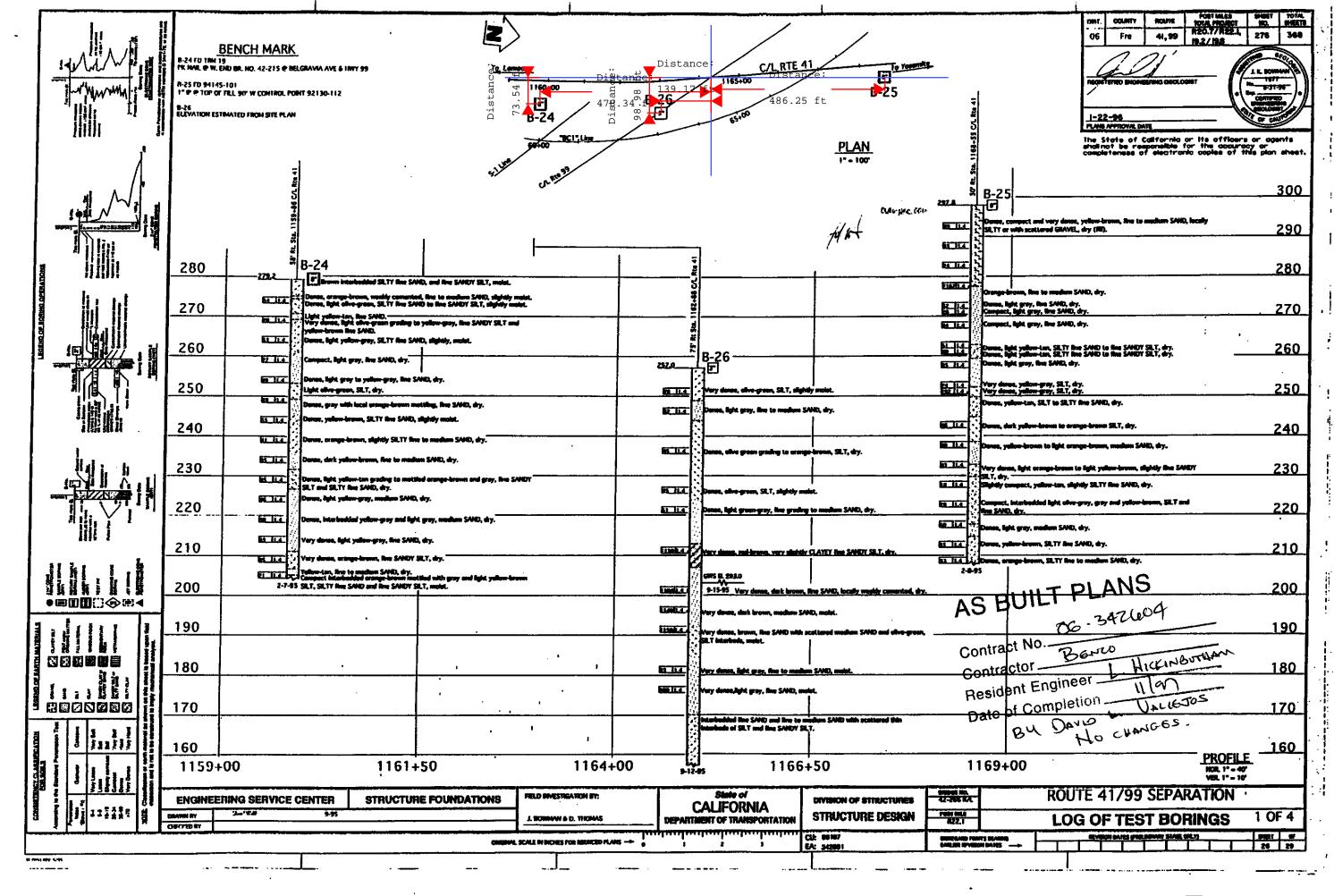


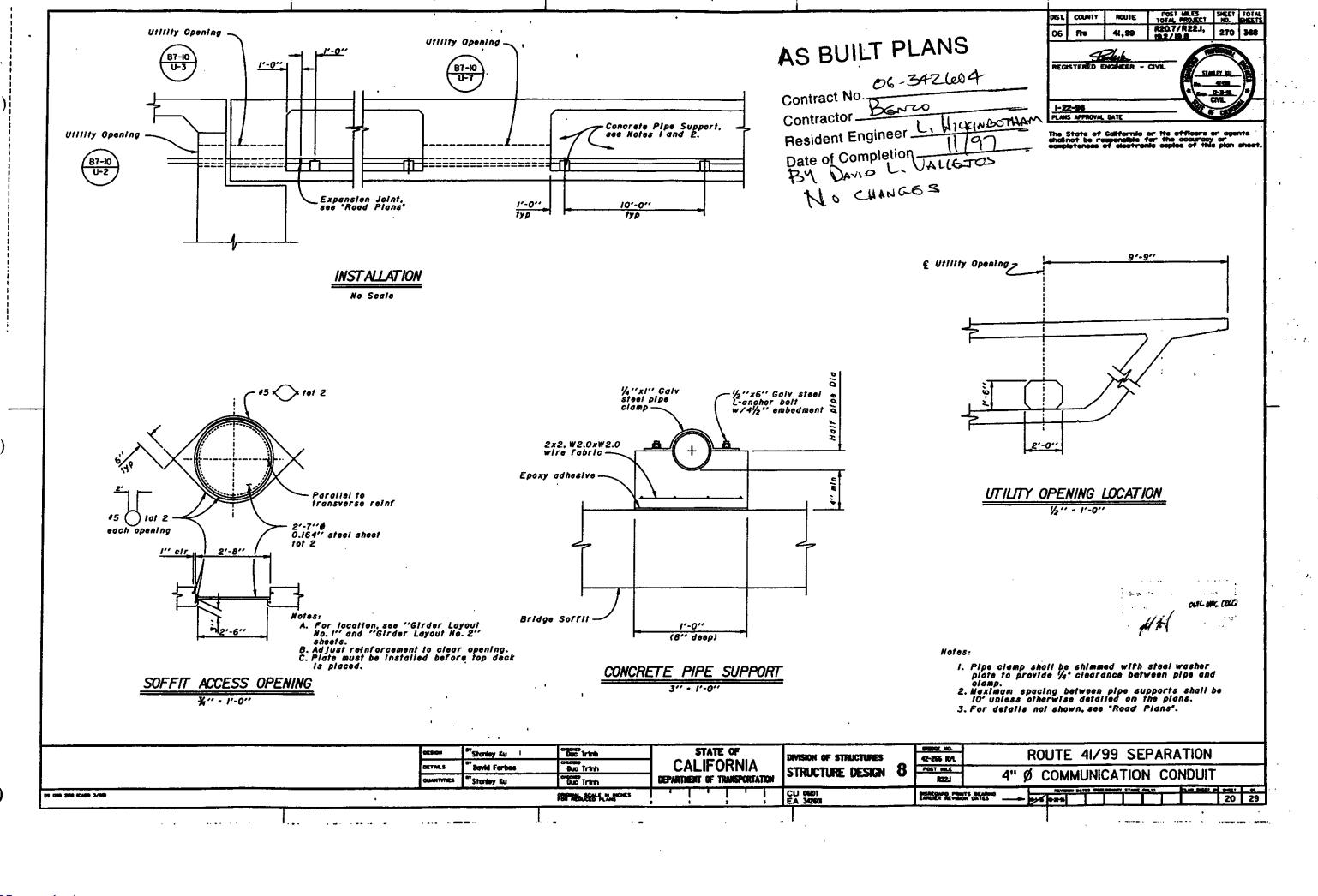


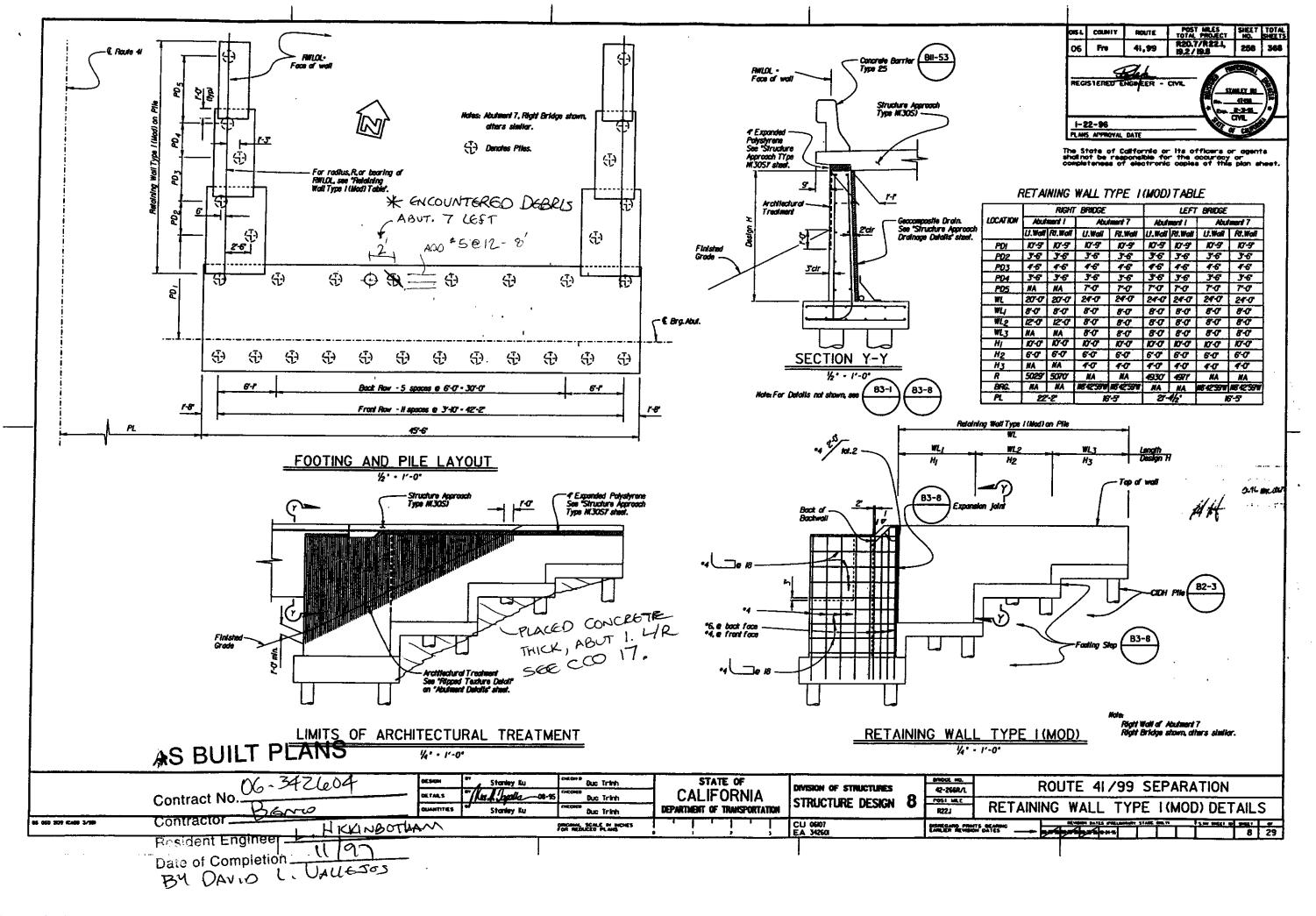


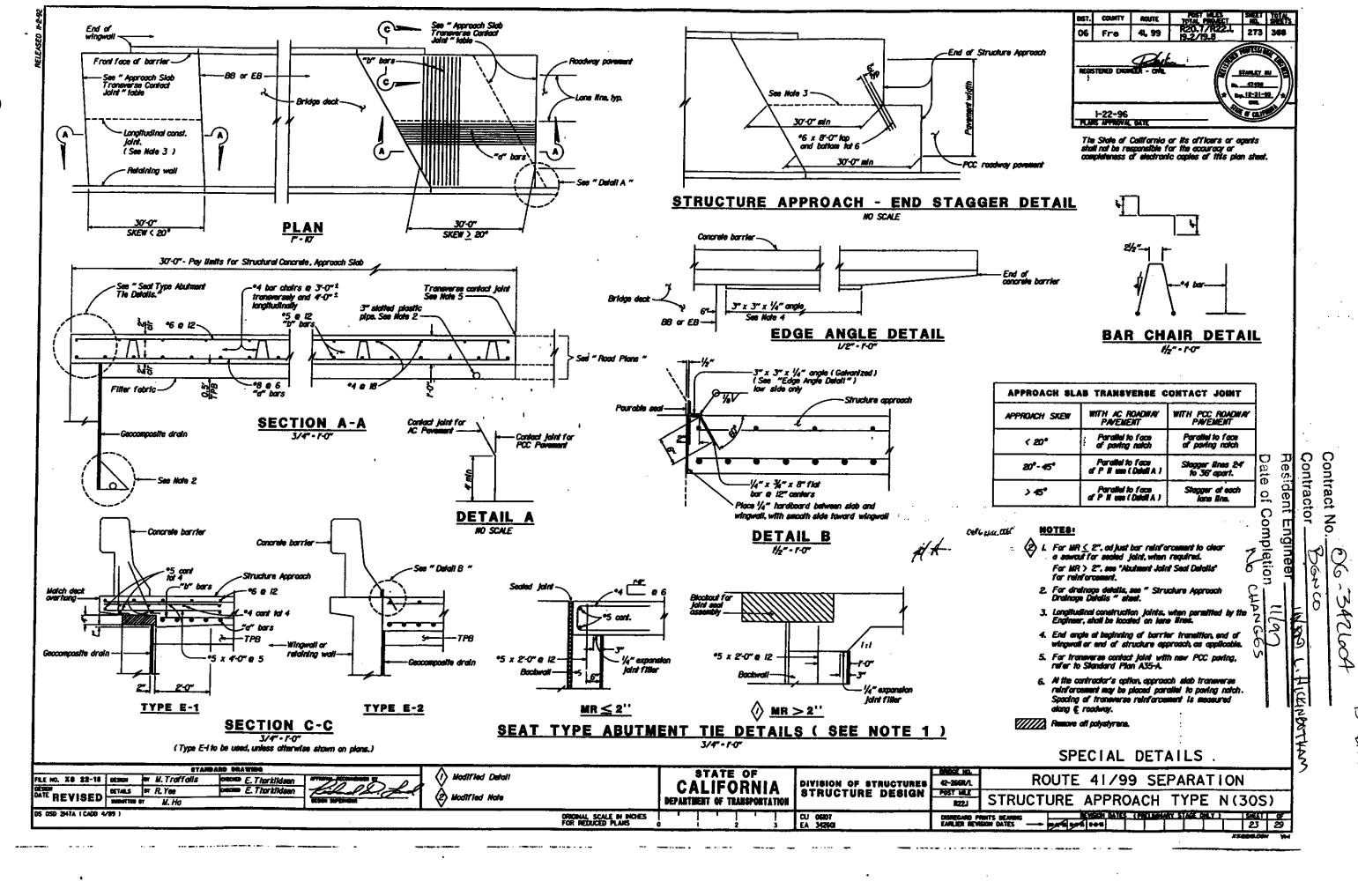










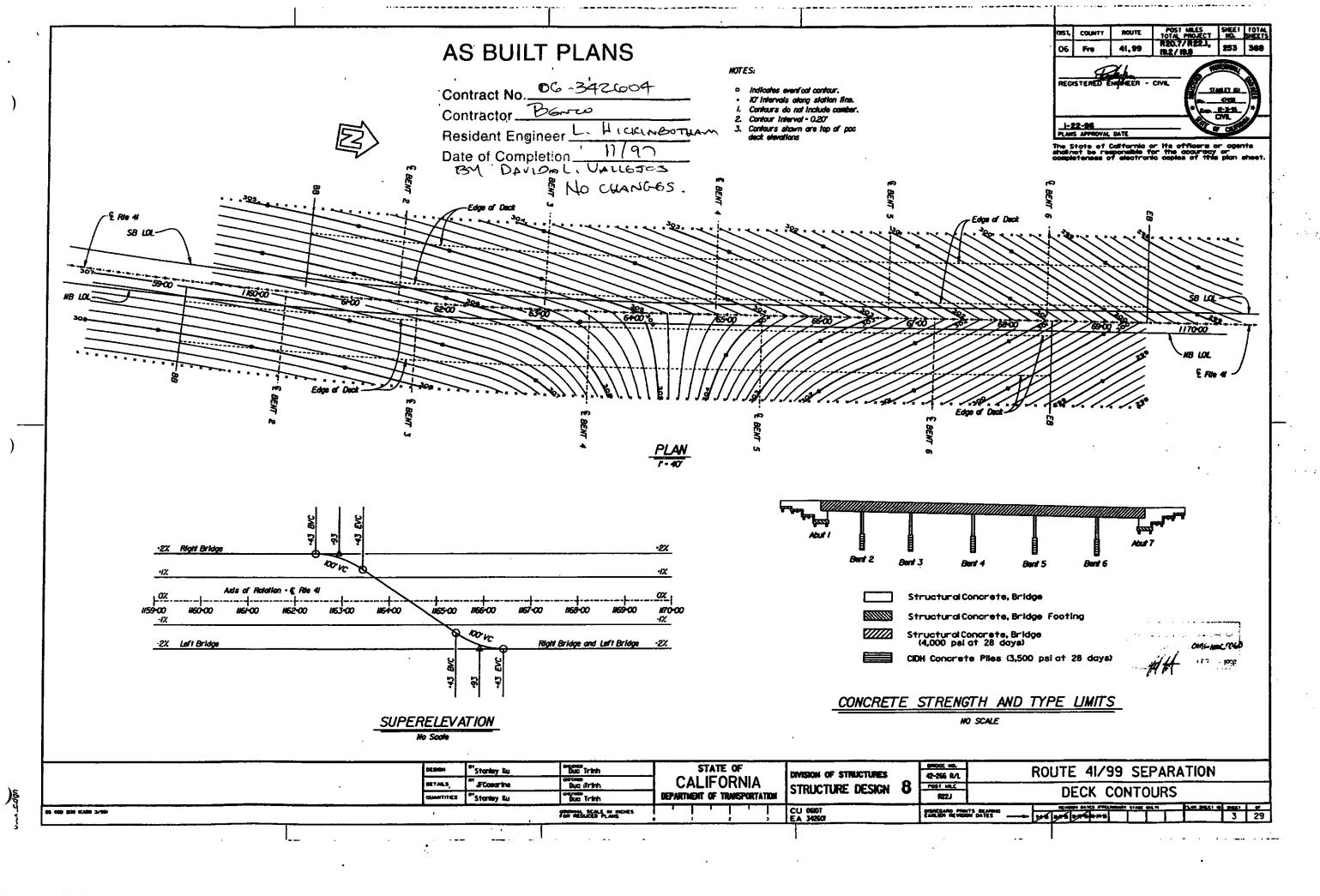


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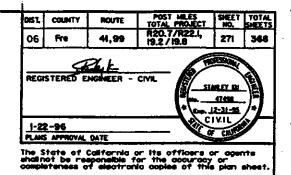
**PLANS** 

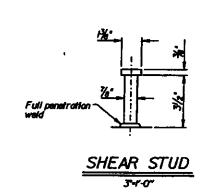


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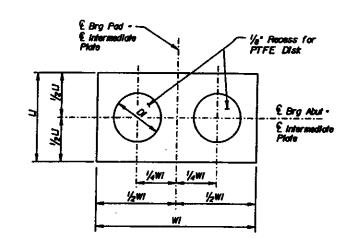
	so	LE PLAT	E	INT	ERMEDI	ATE PLAT	E	MA	SOWRY PL	BEARING PAD				
LOCATION		Ls (in)	Ws (In.)	Ts (In)	U (In.)	WI (In)	TI (In)	DI (In)	Lm (In)	Wm (In.)	Tm (In)	Lb (In)	Wb (In)	Th
Left Bridge	Abut 1 Abut 7	20 20	22 22	2 2	12 12	22 22	11/0	6/2 6/2	12 12	22 22	* *	12 12	22 22	2
Right Bridge	Abut I Abut 7	22 22	22 22	2 2	12 12	22 22	11/8	7	12 12	22 22	* *	12 12	22 22	2

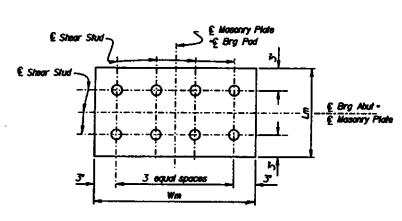
DESIGN	VERTICAL LOAD ON PT'FE BEARING PAD										
LOCA	TION	MAXIMUM	MINIMUM								
Left	Abut I	230 kips	169 kips 153 kips								
Bridge	Abut 7	214 kips									
Right	Abut I	228 klps	167 klps								
Bridge	Abul 7	253 klps	i86 klps								





SECTION L-L

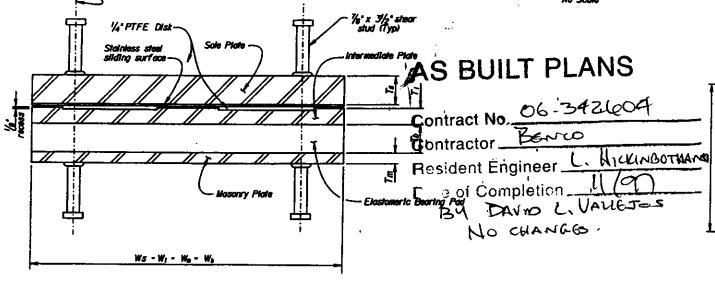


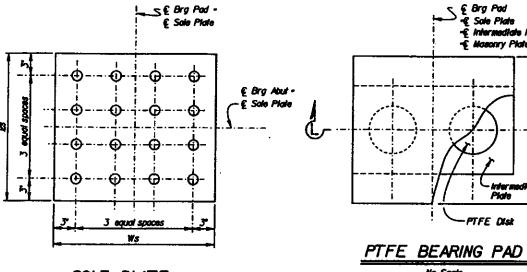


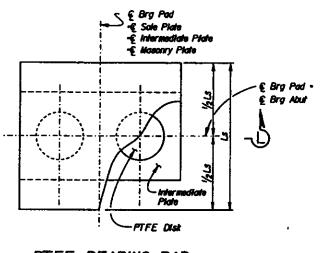
Not all shear stude shown.



MASONRY PLATE

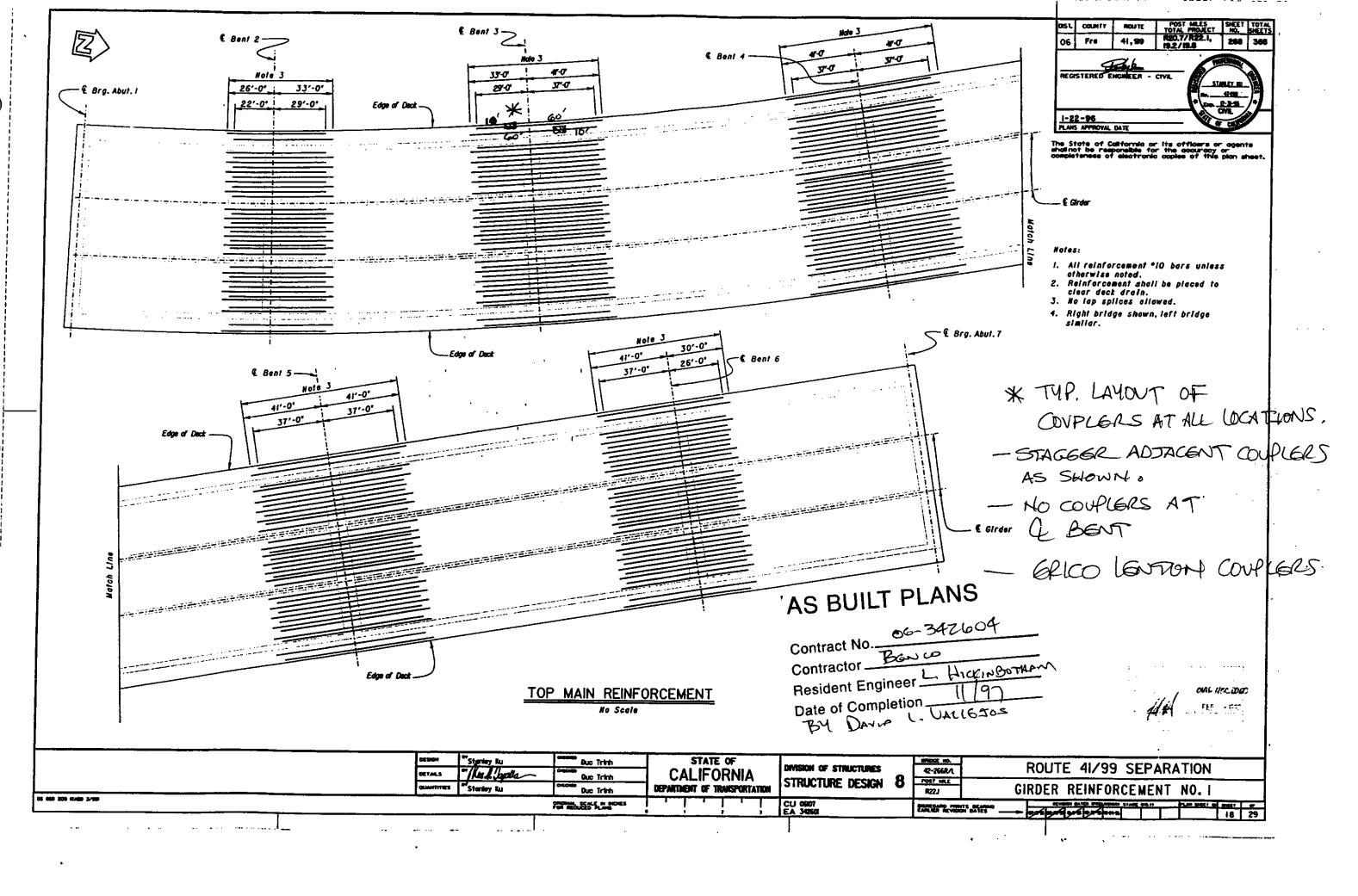


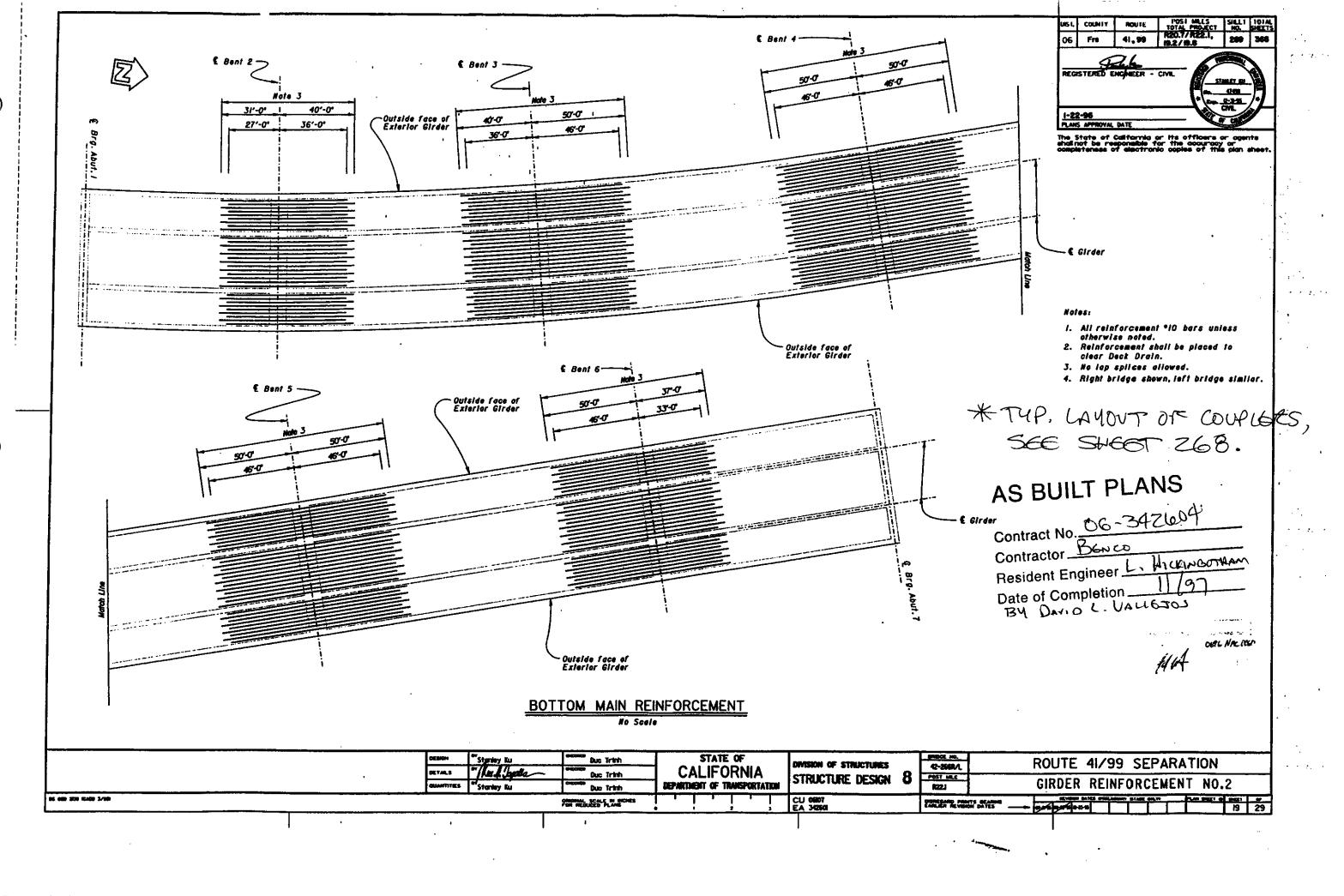


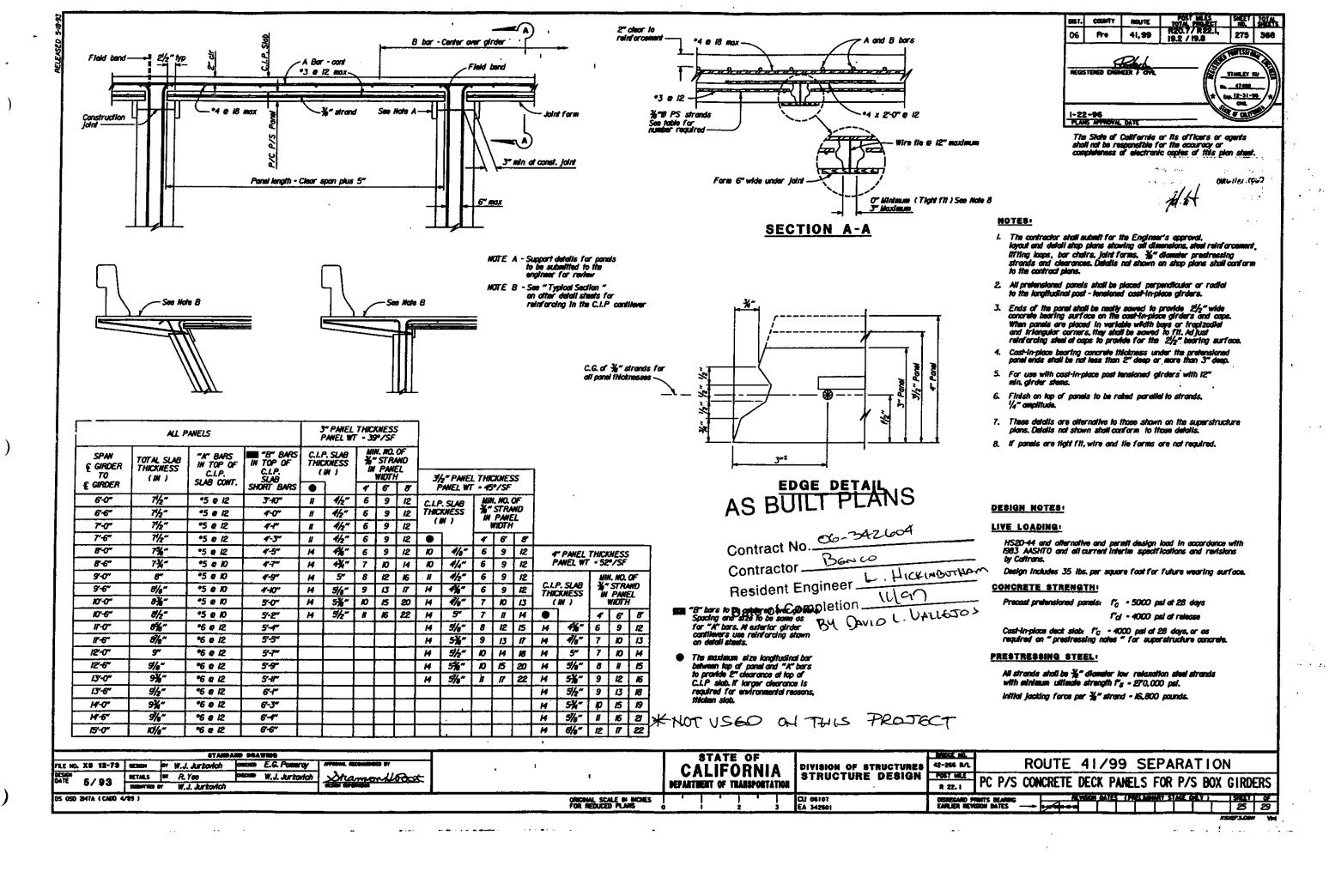


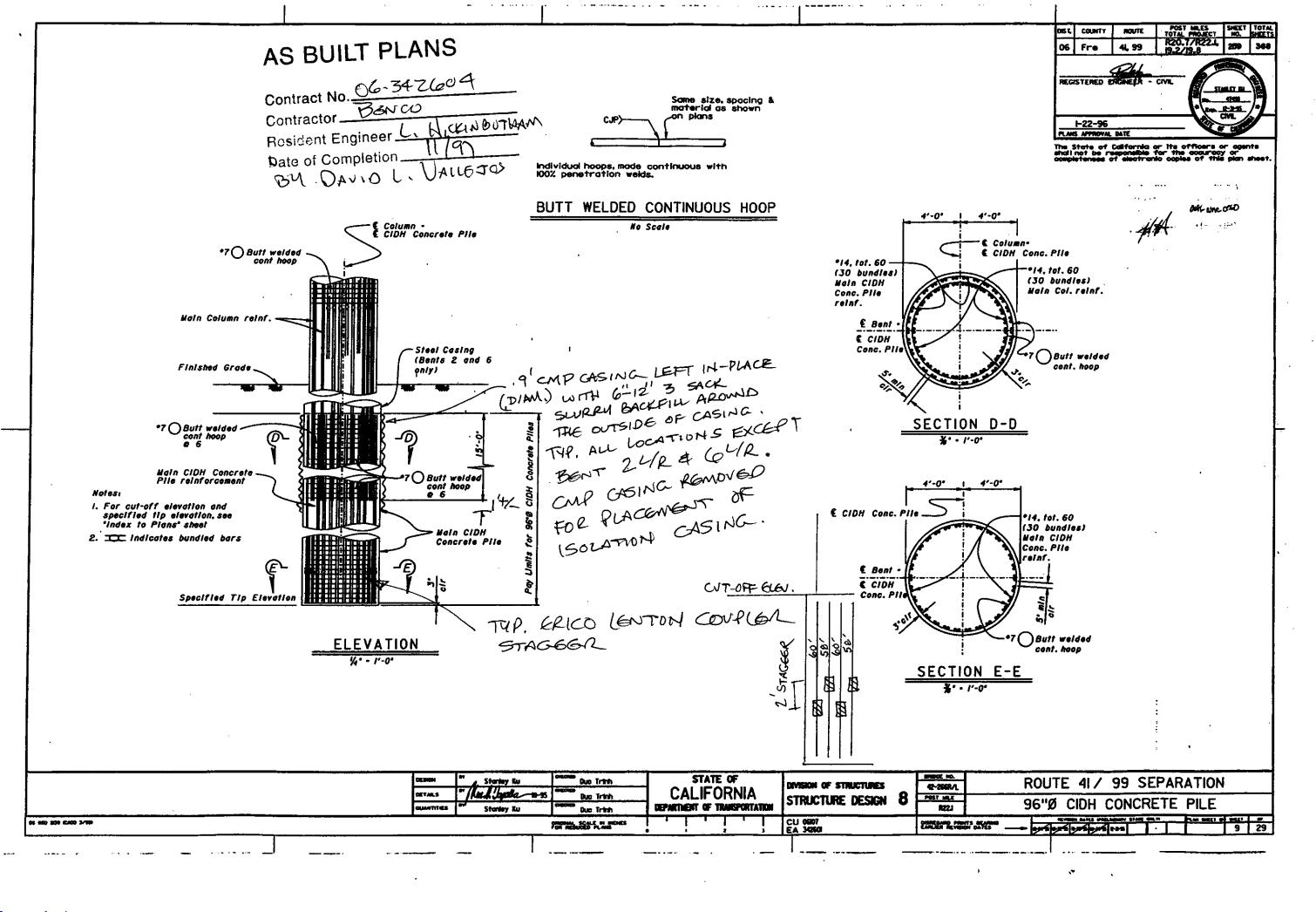
SOLE PLATE No Scale

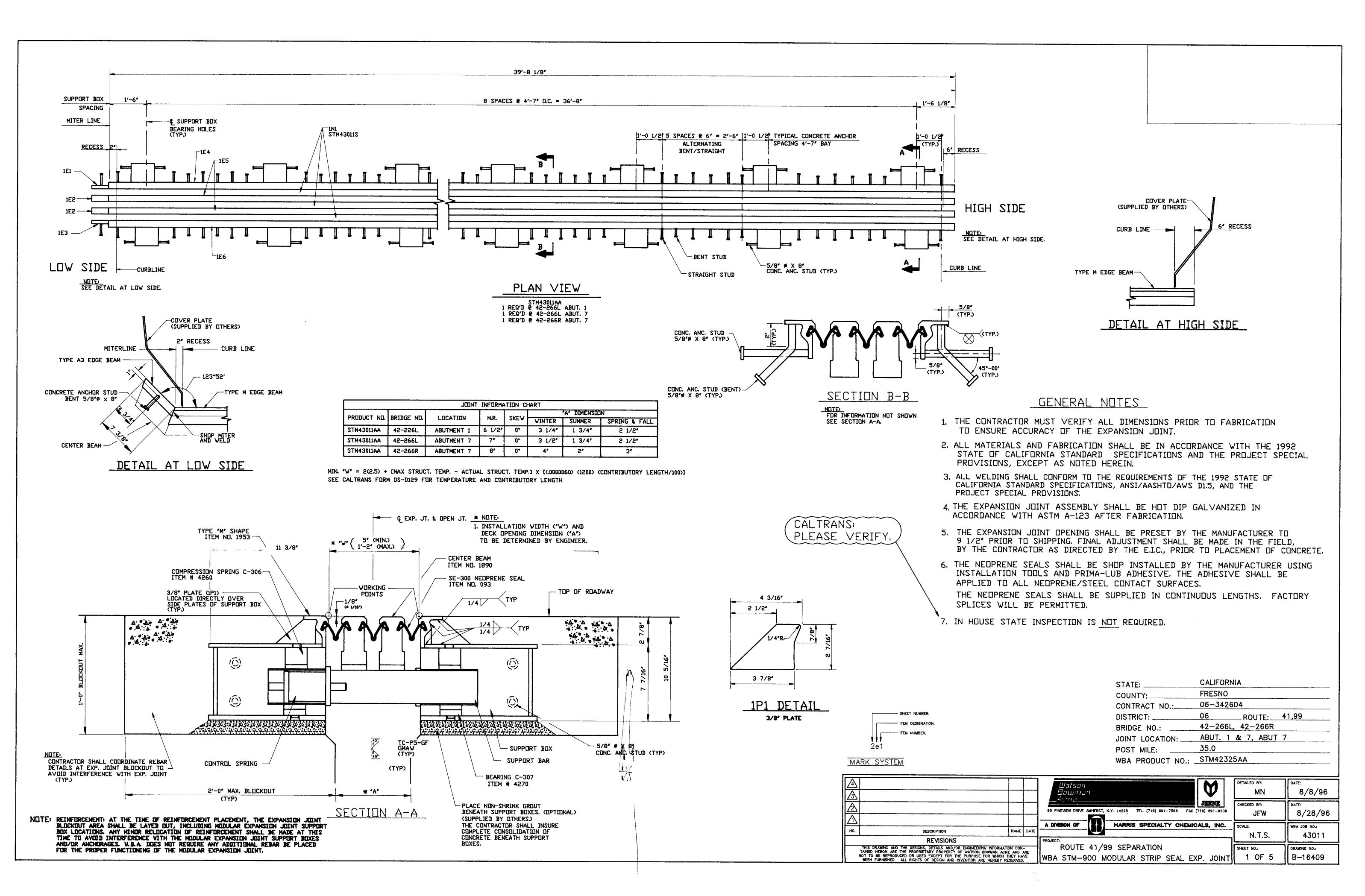
·	9694	Starley Ku	Duo Trinh	STATE OF					SHIDSE HO.	41/99 SEPARATION OVERCROSSING
	DETALS SHORTE LIN DUC Trinh CALIFORNIA STRUCTURE DESIG			_	42-266R/L					
	QUANTITIES	Stanley Ku	Duc Trinh	DEPARTM	MENT OF 1	RANSPORTATI	OH STRUCTURE DESIGN	_	R22. I	PTFE BEARING PAD
66 680 208 (CARP 7/59)			OFFICE PLANE		<u> </u>		CU 06107 EA 342601		EMERY R. VII	173 SECURIOR 183 SECURIOR SECU

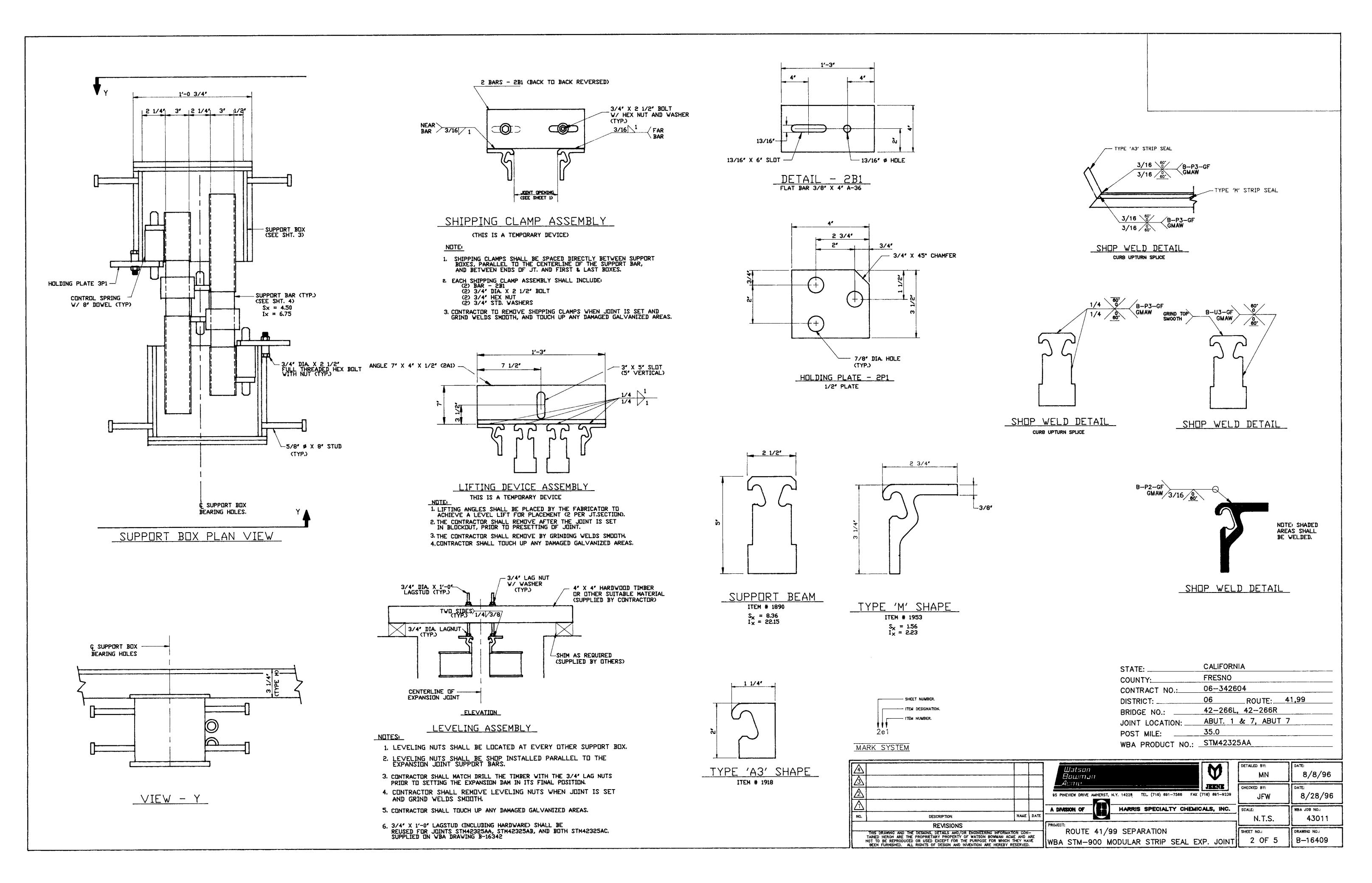


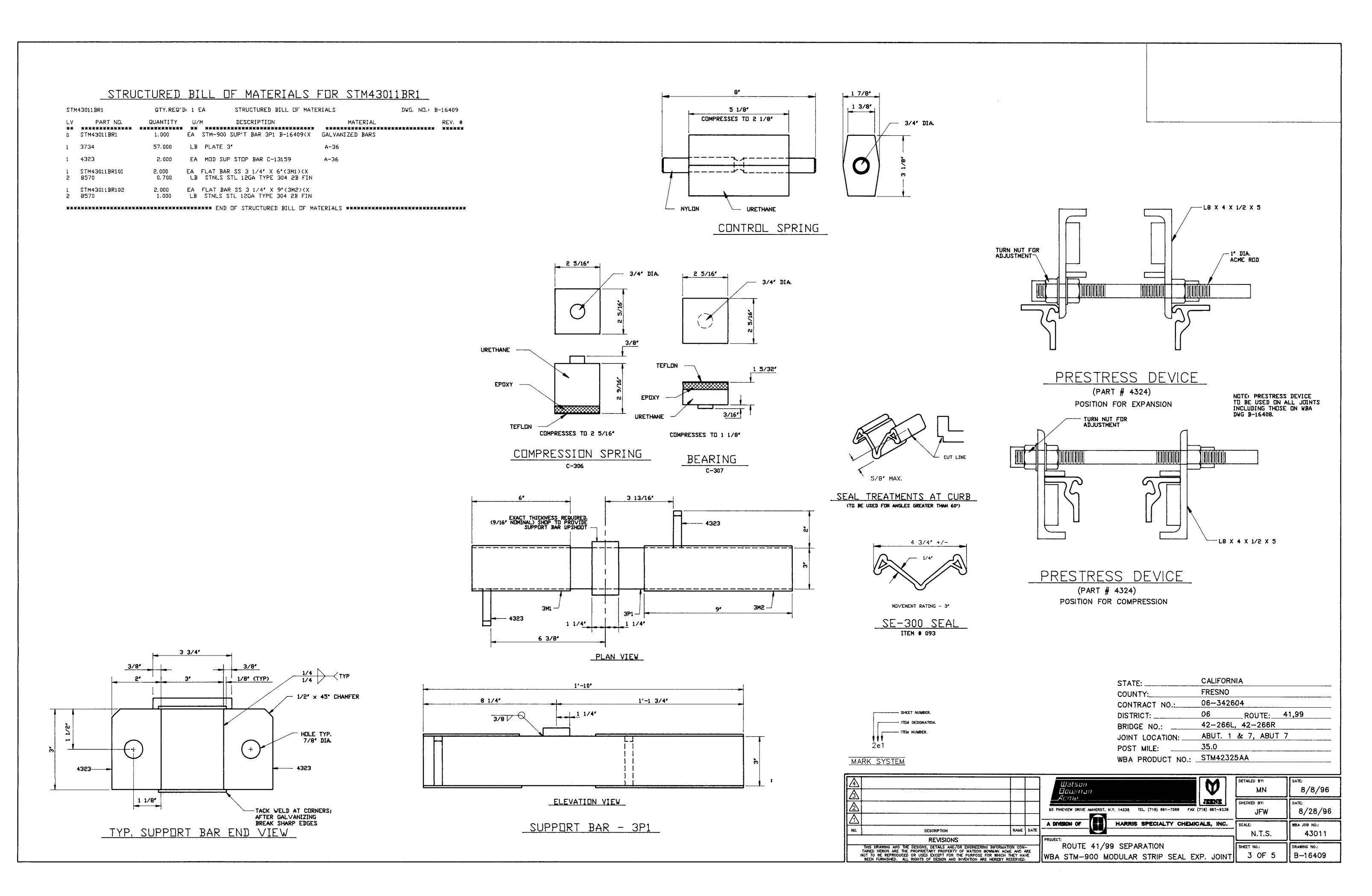


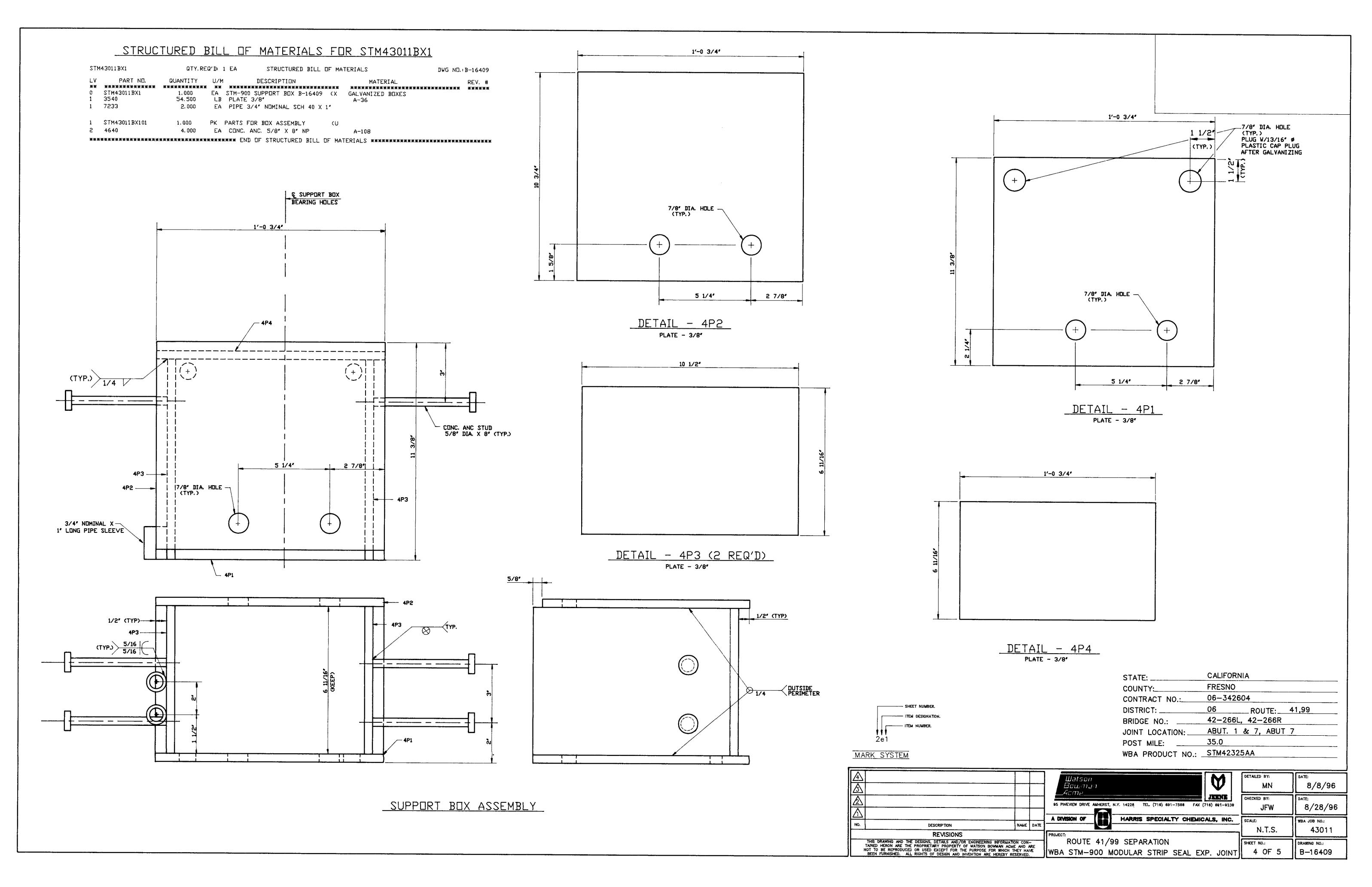












i IM4	43011AA QTYi 3	REQ'D.		STRUCTURED BILL OF MATERIALS	υ <b>∀</b> 6.	. ND: B-1640
LV	PART NO.	QTY	UM	DESCRIPTION	MATERIAL	REV.
0	STM43011AA	1.000	EΑ	STM-900, 40.4 FT (X	SHIPPING LENGTH 40.0' +/-	
					JOINTS ARE GALVANIZED	
1	STM43011S	3.000	EΑ	SEAL FOR SHOP INSTALL		
2	093	42.000	FT	SE-300 STRIP SEAL		
1	STM43011AB03	18.000	EA	HOLDING PLATE (2P1) (X		
5	3560	2.000	LB	PLATE 1/2"	A-36	
1	STM43011AA01	1.000	EA	EXTRUSION ASSEMBLY (X		
2	STM43011AA02	1,000	EA	PROFILES (V		<u> </u>
3	1953	80.000	FT	SSM FM 2-3/4' X 3-1/4' C-11869	A-36	
3	1890	82.000	FT	MOD CB FM 22 C-12183	A-36	
3	1918	2.000	FT	SS A3 FM 1-1/4" X 2" C-11810	A-36	
				O NO THE LATE A LEGISLA	H 30	
2	STM43011BX1	18.900	EA	STM-900 SUPPERT BELX B-16409 (X	A-36	
2	STH43011BR1	18.000	EA	STH-900 SUPPORT BAR B-16409 (X	A-36	
2	STM43011AA03	1.000	PK	PARTS FOR ASSEMBLY (U		
3	4260	36.000	EA	MUD BEARING (UPPER) C-306	COMPRESSION SPRING, URETHANE	
3	4270	36.000	EA	MOD CONTROL BEARING (LOVER) C-307	COMPRESSION SPRING, URETHANE	
3	4321	36.000	EA	MOD CONTROL SPRING BUFFER	C-13751	
3	4320	36.000	EΑ	MOD CONTROL SPRING DOVEL 8"	C-13746	
3	5304	56.000	EA	BOLT 3/4" X 2-1/2" ZP	A-325	
3	8021	56.000	EA	NUT 3/4" ZP	A-325	
3	7602	20.000	EA	WASHER 3/4"	F-436	
3	8084	10.008	EA	NUT 3/4" NL LAG ZP		
3	4640	52.000	EA	CONC ANC STUD ST 5/8" X 8"	A-108	
3	3468	2.000	EA	RUSTOLEUM 2117 GALV. SPRAY PAINT		
2	COMAILOCEMTS	50.000	EA	CONC ANC BENT		
3	4640	1.000	EA	CONC ANC STUD ST 5/8" X 8"	A-108	
2	STM36144AB06	36.000	EA	STIFFNER PLATE (1P1) (X		
3	3540	1,230	LB	3/8" PLATE	A-36	
5	\$70430110E4MT2	20.000	EA	SHIPPING CLAMP (2B1) (V		
	5931	1.260	FT	FLAT BAR 3/8 X 4	A-36	
2	808AII0E4MT2	2.000	EA	LIFTING ANGLE (2A1) (V		
	4130	1.300	FT	ANGLE 7" X 4" X 1/2"	A-36	

# MATERIAL SPECIFICATIONS

STEEL EDGE & SUPPORT BEAMS - All beams are made of ASTM A-36 grade steel and have grooves which grip the neoprene locking seal,

NEOPRENE LOCKING SEAL - Seals shall be in accordance with ASTM D-2628. The neoprene locking seal is bonded to the steel beams with Prima-Lub Adhesive. The neoprene seal is designed to absorb all joint movements. The physical properties of the locking seal are as follows:

PROCEDURE (D-412)	REQUIREMENT 2000 PSI
(D-412)	250%
(D-2240) M□D.	55 - 70
(D-395) Method B Mod.	40%
(D-573)	20% 20% 0 to +10
	45%
(D-1149)	No Cracks
	(D-412) (D-412) (D-2240) M□D. (D-395) Method B Mod. (D-573)

COMPRESSION SPRING C-306 - This compression spring is composed of urethane, epoxy and 3/64" thick teflon sheet. The compression spring sits on top of the support bar. The physical properties of the unethane are:

Elongation at Break       (ASTM D-412)       425%         Tensile Strength       (ASTM D-412)       6500 F         100% Modulus       (ASTM D-412)       1200 P         300% Modulus       (ASTM D-412)       2400 F         Tear Strength       (ASTM D-470)       110 PL	sile Strength	(ASTM D-412)	6500 PSI
	Modulus	(ASTM D-412)	1200 PSI
	Modulus	(ASTM D-412)	2400 PSI
	Strength	(ASTM D-470)	110 PLI
Rebound Resilience (ASTM D-2632) 40%	ound Resilience		

CONTROL SPRING - The control spring which is located between the support bars act to equalize the expansion of each seal. The control spring is made of URETHANE.

STAINLESS STEEL - Stainless steel is used on the sliding surfaces of the support bar that contact the teflon surface of the bearing and compression spring. The stainless shall be ASTM A167, Type 304 No. 2B finish.

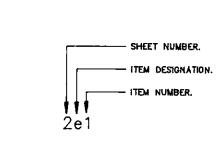
PRIMA-LUB ADHESIVE - Prima-lub Adhesive is used to bond the neoprene locking seal to the steel extrusions. This adhesive shall be a one-part moisture curing polyurethane and hydrocarbon solvent mixture with the following physical properties:

Average Weight per Gallon 8.5 lbs +/- 10% Sollds Content 72% (mln.) Adhesive to remain workable From 5-120°F Film Strength 2000 PSI (min.) Elongation at room temperature 350% (mln.) Flash Point (seta closed cup) over 100°F

BEARING C-307 - The bearing is composed of unethane, epoxy and 3/64" thick teflon sheet on which the support bar slides on. The physical properties of the urethane are:

### INSTALLATION PROCEDURE

- STEP 1. VERIFY THE DIMENSIONS OF THE BLOCKOUT A-A ON SHEET 1. CORRECT AS NECESSARY.
- STEP 2. LIFT AND PLACE EXPANSION JOINT INTO BLOCKOUT. WHILE JOINT IS SUSPENDED, INSTALL LEVELING DEVICES AND ADJUST TO PROPER GRADE AND ELEVATION.
- STEP 3. CHECK JOINT FOR ALIGNMENT WITH CURBS. REMOVE LIFTING ANGLES AND GRIND
- WELDS SMOOTH, LOOSEN BOLTS AT SHIPPING CLAMPS IF REQUIRED TO ADJUST THE EXP. JOINT.
- STEP 4. PRESET THE EXPANSION JOINT OPENING AS DIRECTED BY THE FIELD ENGINEER.
- TIGHTEN BOLTS AT SHIPPING CLAMPS. STEP 5. PRIOR TO PLACEMENT OF CONCRETE, ALL PRESTRESS DEVICES SHALL BE REMOVED.
- DEVICES ON TOP OF THE JOINT MAY REMAIN IF THEIR LOCATION WILL NOT INTERFERE WITH CONCRETE PLACEMENT OR EXPANSION JOINT PERFORMANCE. STEP 6. TEMPERATURE AND JOINT OPENING SHOULD BE CHECKED FOR ANY DISCREPANCIES
- FROM INITIAL ADJUSTMENT.
- STEP 7. CONTRACTOR SHALL AT THIS TIME HAVE REQUIRED FORMWORK IN PLACE.
- STEP 8. ALL CONCRETE PLACEMENT SHALL BE IN ACCORDANCE WITH THE SPECIFICATIONS.
- STEP 9. UPON COMPLETING CONCRETE PLACEMENT OPERATIONS, LOOSEN NUTS AT SHIPPING CLAMPS, THE ENGINEER SHALL DETERMINE WHEN REMOVAL OF THE LEVELING DEVICES WILL BE PERMITTED.
- STEP 10. CONTRACTOR SHALL REMOVE ALL TEMPORARY DEVICES FROM TOP OF JOINT AND TOUCH UP ALL AREAS OF DAMAGED GALVANIZING.



MARK SYSTEM

CALIFORNIA STATE: \_\_ FRESNO COUNTY: 06-342604 CONTRACT NO .:\_ 06 ROUTE: 41,99 DISTRICT: \_\_ BRIDGE NO .: \_\_ 42-266L, 42-266R ABUT. 1 & 7, ABUT 7 JOINT LOCATION: \_\_\_ 35.0 POST MILE: WBA PRODUCT NO .: STM42325AA

<u>A</u>			Watson Bow nan	DETAILED BY:	DATE: 8/8/96
			95 PINEVIEW DRIVE AMHERST, N.Y. 14228 TEL (718) 691-7566 FAX (718) 691-9239	CHECKED BY:  JFW	DATE: 8/28/96
NO. DESCRIPTION  REVISIONS	NAME I	DATE	A DIVISION OF HARRIS SPECIALTY CHEMICALS, INC.	scale: N.T.S.	WBA JOB NO.: 43011
THIS DRAWING AND THE DESIGNS, DETAILS AND/OR ENGINEERING INFORMATION TAINED HERON ARE THE PROPRIETARY PROPERTY OF WATSON BOWMAN ACME NOT TO BE REPRODUCED OR USED EXCEPT FOR THE PURPOSE FOR WHICH THE BEEN FURNISHED. ALL RIGHTS OF DESIGN AND INVENTION ARE HEREBY RES	AND AR		ROUTE 41/99 SEPARATION WBA STM-900 MODULAR STRIP SEAL EXP. JOINT	SHEET NO.: 5 OF 5	DRAWING NO.: B-16409